TG12.1 – Civil/Sitework at Grade/Ground Level Landscaping

Questions are numbered in the order received. Numbers missing in the sequence either have been answered in a previous response set or will be answered in a future set.

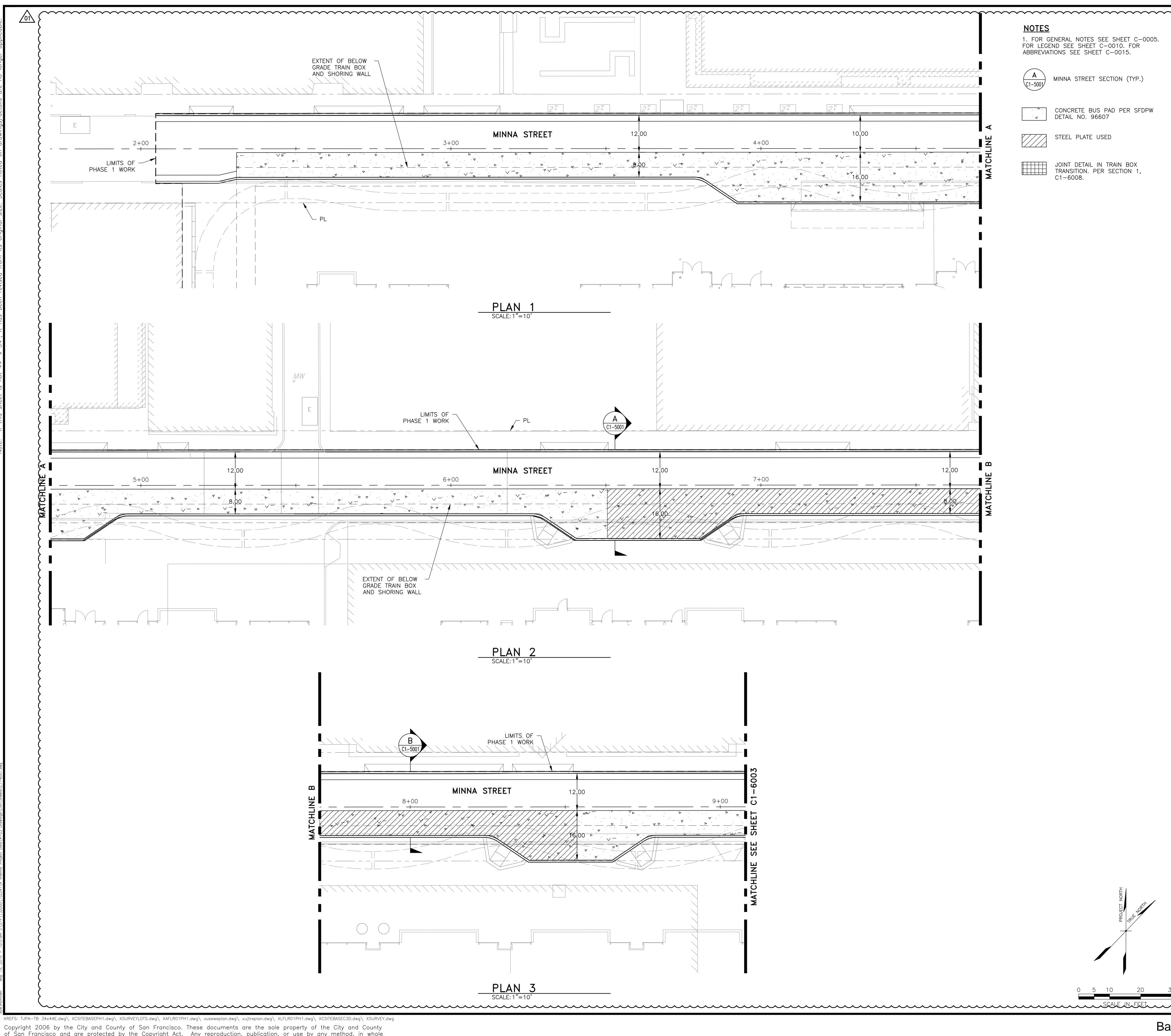
Question No.	Submission Date	Drawing No.	Document/ Spec. No.	Question	Response
TG12.1- 001	5/7/2015	C1- 2000s, C1- 4000s, C1- 5000s		Plan Sheets C1-200x, C1-400x, C1-500x (x = various numbers) graphically show curbs without gutter sections. This is consistent with Note 4 on the various C1-500x sheets, which refers to 6" Curb per SFDPW Std #87169. But the Paving Plans (Sheets C1-600x) graphically indicate that the curb adjacent to the typical street buildup sections (C1-6008) is a curb and gutter section, as depicted on SFDPW Std #87170, rather than curb-only. Please clarify.	Curb and gutter to be provided as shown graphically in the Paving Plans (Sheets C1- 600x). Refer to SFDPW Std #87170 for curb and gutter sections.
TG12.1- 006	5/7/2015	C1-6008 & C1- 5001-C1- 5005		Plan Sheet C1-6008 shows that 1/4" steel plate is to be used in lieu of 6" of sub-base when 6" isn't available. How do we determine where the steel plate will be required? Plan Sheets C1-5001 thru C1-5005 indicate that much of the street area over the box is very close to this 6" minimum, and the cost differential between steel and sub-base is significant.	For clarification of steel protection plate locations, please refer to attached Civil Sketches QBD006-01 through QBD006-05. Steel plate is not part of TG12.1 scope.
TG12.1- 007	5/7/2015	C1-6008 & C1- 5001-C1- 5005		In areas referenced in drawings that require steel plate, what (if anything) is placed between the steel plate and concrete base?	No additional construction material is required to be placed between the steel plate and the concrete base.
TG12.1- 009	5/7/2015			Is special fill (i.e. lightweight) required for the permanent street fills between the TOTB and pavement sections at First, Fremont, and Beale streets?	There is no special fill/lightweight fill required along First, Fremont, and Beale streets between TOTB and the pavement section; standard fill is acceptable.
TG12.1- 012	5/7/2015	C1-6008, C1-5002		The permanent roadway sections at First, Fremont, and Beale streets (per C1-6008) include 2" of AC over variable thickness concrete base and 6" of sub-base, over an undefined material under the sub-base. Also, Page 10 of 13 (in TG12.1 document) includes a note to "include section of structural concrete per detail C1-5002" There is no structural concrete noted on C1- 5002, or the noted/referenced C1-6008 sections. Should this be "concrete base"? This section does not match the sections placed in the Bridge Removal Plans.	The "TG12.1 First and Fremont St. Removal" document is for reference only. The note to "include section of structural concrete per detail C1-5002" on Pg. 10 is meant to refer to the "concrete base" callout as shown on C1- 6008.

TG12.1- 014	5/7/2015			What base and/or sub-base material is to be placed under the Bus Pad Concrete in the Bus Plaza? Almost all other paved areas that are constructed on the Train Box are built on Geosynthetic material. Is the same required here?	In the Bus Plaza, follow C1-6008, Detail 6, Note 1. "Extend Concrete Pavement to Top of Building Insulation"; therefore, no sub-base is used as concrete extends to top of train box.
TG12.1- 021	5/13/2015		Sections 2.3 and 2.4 03 33 12	Technical Specification Section 03 33 12, Landscape Cast-in-Place Concrete, Paragraphs 2.3.D and 2.4.A.2, cite specifications for Micro-reinforcement in the concrete (as well as reinforcing bars and welded wire fabric). The plans show reinforcing bars but not mesh, nor is micro-reinforcement noted. Is micro- reinforcement required in the paving concrete?	Yes, micro-reinforcement is required in the concrete paving.
TG12.1- 022	5/13/2015	L1-7310 Detail 10, L1- 2302 - L1-2307		Plan Sheet L1-7310 shows a 9" transition slab beneath the Pedestrian or Vehicle Paving at the "On-Building" and "On-Grade" junction. These section cuts are only shown on Sheets L1-2302 and L1-2303 (west end and portion of SW side). There are similar situations elsewhere (i.e. north side and east end), are these transition slabs required here also? If so, please provide limits/dimensions.	No other transition slabs are required except the locations indicated on the drawings.
TG12.1- 023	5/13/2015	L1-2302 thru L1- 2307		The Legend on Plan Sheet L-0002 includes several symbols for "Expansion Joints" (including a "non-doweled expansion joint" and "Doweled Cold Joint." There's only about 20 doweled cold joints shown on Sheets L1- 2302 thru L1-2307, and no non- doweled expansion joints. It appears that all expansion joints are doweled, please confirm.	Non-doweled expansion joints are typically at building edge. Please refer to material plans.
TG12.1- 024	5/13/2015		Section 3.12 03 33 12	Technical Specification Section 03 33 12, Landscape Cast-in-Place Concrete, Paragraph 3.12, notes that Staining is required. (1) The concrete has integral color: please confirm that integral color AND staining is required; (2) If required, Paragraphs 2.3 (Materials) nor 3.12 (Staining) note what material is required, please provide; (3) 3.12 states that 3 coats are to be applied at the coverage rate recommended by the manufacturer. Is each coat to be at that rate, or just the total of the 3 coats?	Staining is not required. Please refer to Specification Section 03 33 12, Schedules 3.16 for concrete colors and finishes.

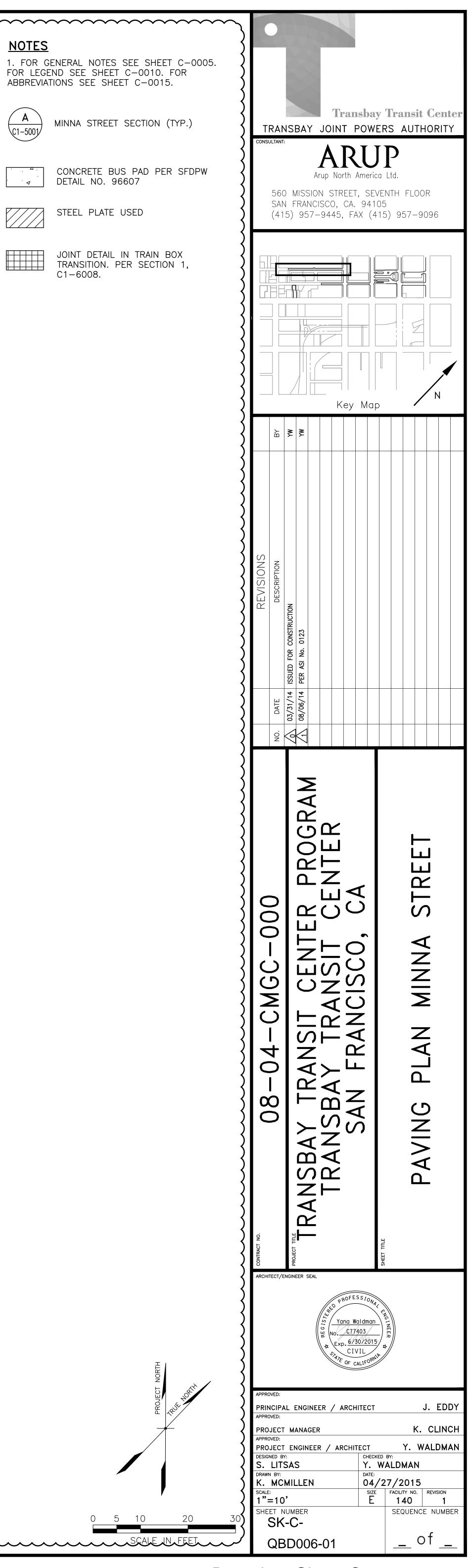
TG12.1- 025	5/18/2015			The columns/shafts for the temporary bridges to be removed are nominally 48" diameter, but due to required construction methods (drilled shaft) they are typically larger. What is the maximum diameter of the columns to be removed? Will the TG12.1 Trade Subcontractor be required to perform any chipping to allow removal of these columns, as detailed in the Temporary Bridges Removal Narrative? If so, how much?	CIDH drilled piers vary in diameter as they were installed without sleeves; chipping will be necessary if the TG12.1 Trade Subcontractor chooses to cut and remove through the blockout. A jobsite visit is strongly recommended to assess the extent of the chipping as each of the CIDH piers vary in size. See attached submittals for reference.
TG12.1- 026	5/18/2015	L1-2330 - L1- 2332, L1-2302 - L1- 2304		Plan Sheets L1-2330 through L1-2332 show ventilation manholes and ventilation grates within lift out lids, with notes to "SCD" (See Civil Drawings). No details are provided in the Civil or Landscape Drawings. Similarly, there are "Fuel Intakes" (per Legend) shown on Plan Sheets L1-2302 & L1-2304 with the note "SED" (see Electrical Drawings). No details were found. Please provide details for all three of these features as they relate to the work of TG12.1.	The reference notes in drawings have been amended to state "SAD." Refer to architectural details 9/A1-8717 and 1 & 4/A1- 9239 for these elements. See attached sketches SKLA-432.1 through SKLA-432.4 for clarification.
TG12.1- 027	5/18/2015		CDSM Cutoff Procedure	The Supplemental Documents list a "CDSM Cutoff Narrative." This file isn't in the referenced BOX link, but upon our prior request we were sent a 7-page document, titled "CDSM Cutoff Procedure." This document includes a narrative and sketches, and noted that others (not TG12.1) will have prepared the beam for future cutoff by providing a beveled 0" to 6" (at the building- side flange) cutout extending 3" minimum into the flange to facilitate its future cutoff. A recent site visit indicates at least some of these cuts are not prepared as noted, with only a thin (<1") cut made at the flange, not the 6" cutout shown. It is impossible to see most of the cuts as they have been patched over. Please confirm what condition the existing shoring beams will be in prior to TG12.1 starting their removals.	The TG07.2 Trade Subcontractor is responsible for the following (from TG07.2 Exhibit A): Cut entirely through the interior flange and 2" into the web of steel soldier pile beams with a cutting torch around the entire CDSM wall perimeter. Grind smooth the edge of the cut surface as necessary to receive WPM-1 waterproofing system, Grace Preprufe. Assume that the cutting of the pile beams will occur between 2' and 8' below the finish grade surface; the precise elevations for the cutting of the pile beams will be determined by future design requirements. The field conditions may differ from what was shown in the TG07.2 Exhibit A/ CDSM Cutoff Procedure.
TG12.1- 028	5/18/2015		CDSM Shoring Wall Removal, Exhibit A	There is currently dewatering pipe, temporary electrical facilities, and other obstructions within or immediately adjacent to the shoring wall alignment. Will others remove these temporary obstructions (excluding the temporary K-Rail and perimeter fencing) prior to the start of the TG12.1 CDSM Shoring Wall Removal operations?	All electrical, dewatering, and miscellaneous conduit and/or PVC lines will be removed by other contractors prior to the TG12.1 Trade Subcontractor's scheduled activities. The removal will be coordinated by Webcor/Obayashi Joint Venture field superintendent.

TG12.1- 029	5/18/2015		No Information has been provided on the existing Beale Street Temporary Bridge. Please provide information regarding this structure and its required removal phasing, or confirm that the work required is accurately depicted in the documents already available.	The removal of the Beale Street Bridge will follow a similar process as First and Fremont Street. See attached submittal for reference.
TG12.1- 030	5/18/2015	C1-5002 thru C1- 5004	What fill material(s) are required under and outside of the "typical street buildup" section (C1-6008) over the Train Box at First, Fremont and Beale streets? We can't find where this is shown in the plans or an applicable specification. The Temporary Bridge Removal scenarios provided show recycled AB, CDF, "structural fill per specifications" and/or extruded polystyrene, but it's unclear whether these are just temporary sections and have to be replaced when final construction is complete, and if so, with what? Please clarify.	In the permanent condition under the "typical street buildup," there is no special fill/lightweight fill required along First, Fremont, and Beale streets between TOTB and the pavement section. Backfill (standard fill) shall be used.
TG12.1- 032	5/18/2015		Trade Subcontractor requests a two week bid extension to 6/23. Due to the complexity of the project, specifically regarding the temporary bridges removal, and Trade Subcontractor's focus on other recent Transbay Transit Center trade package bids, more time is needed to competently and competitively bid this package. Thank you.	See Addendum 2 for revised Package Timeline.

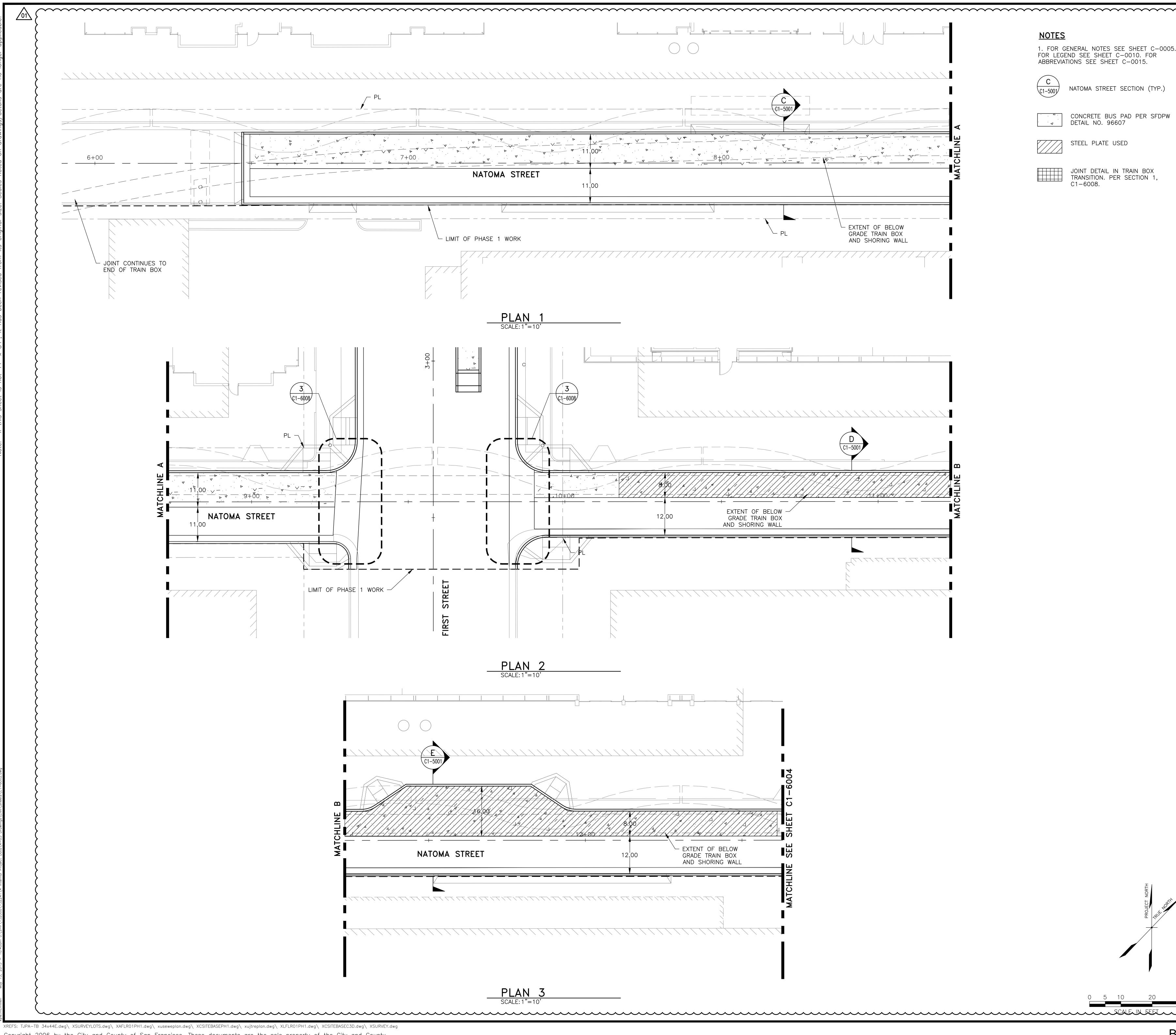




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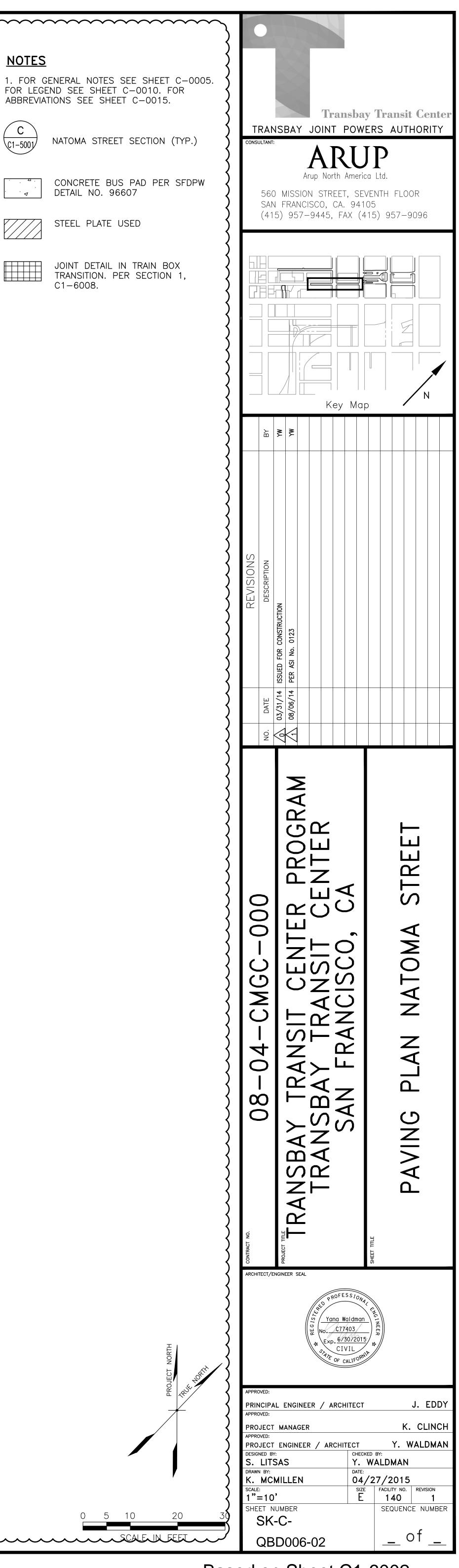


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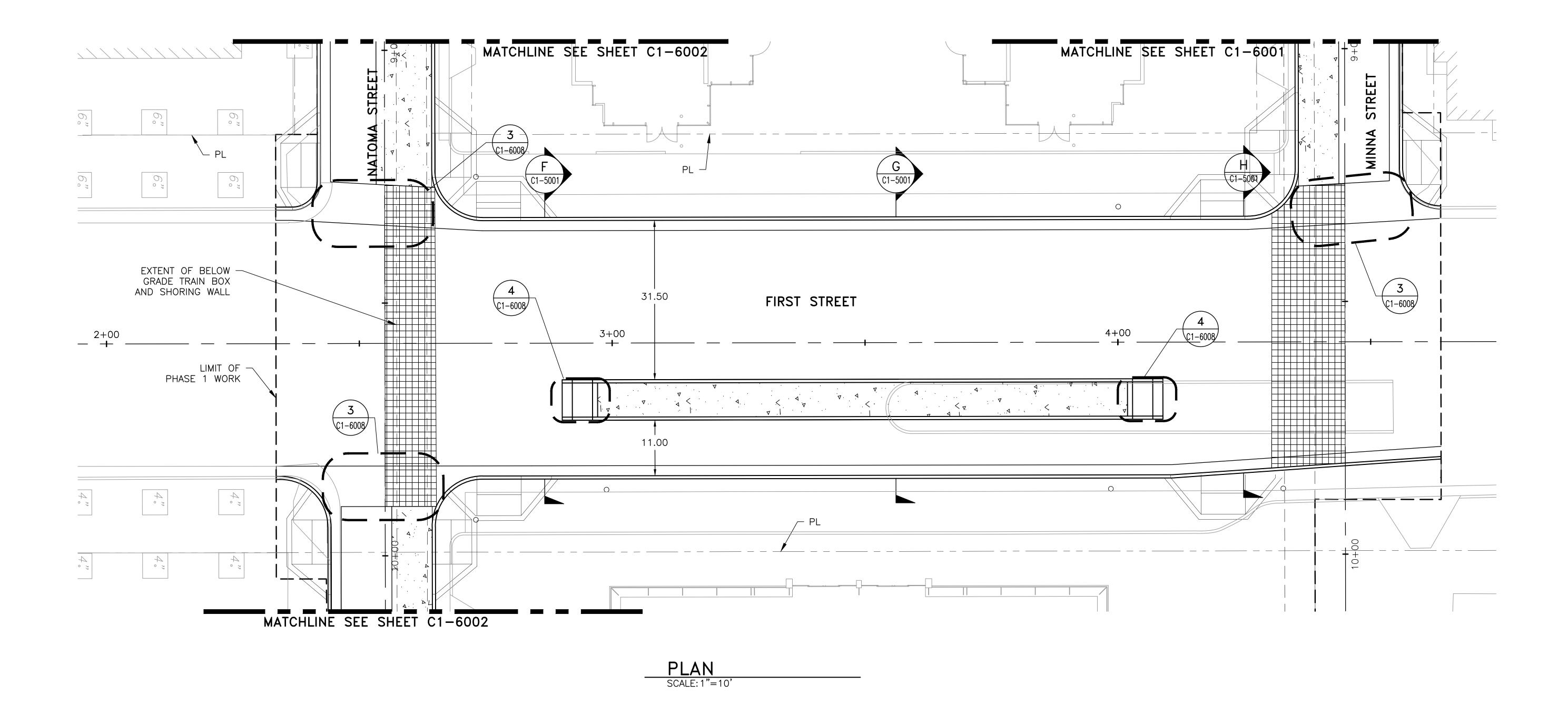


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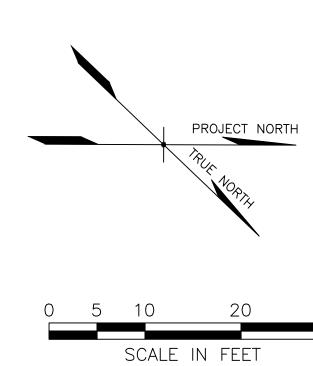


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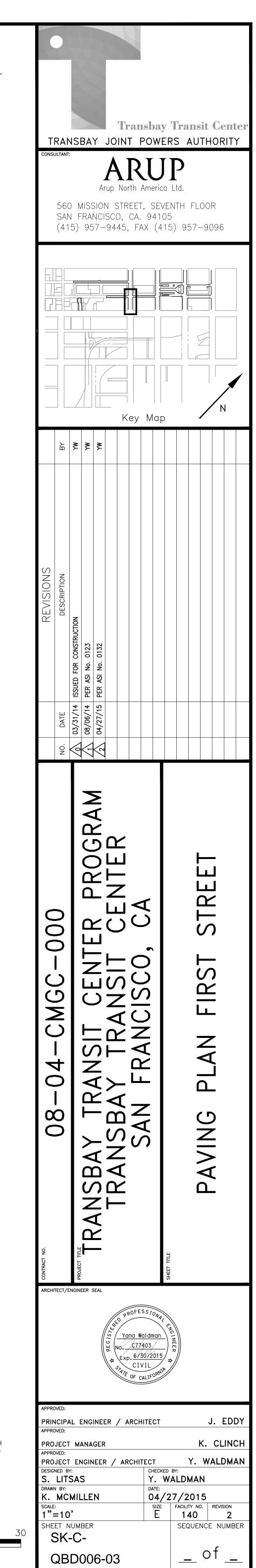


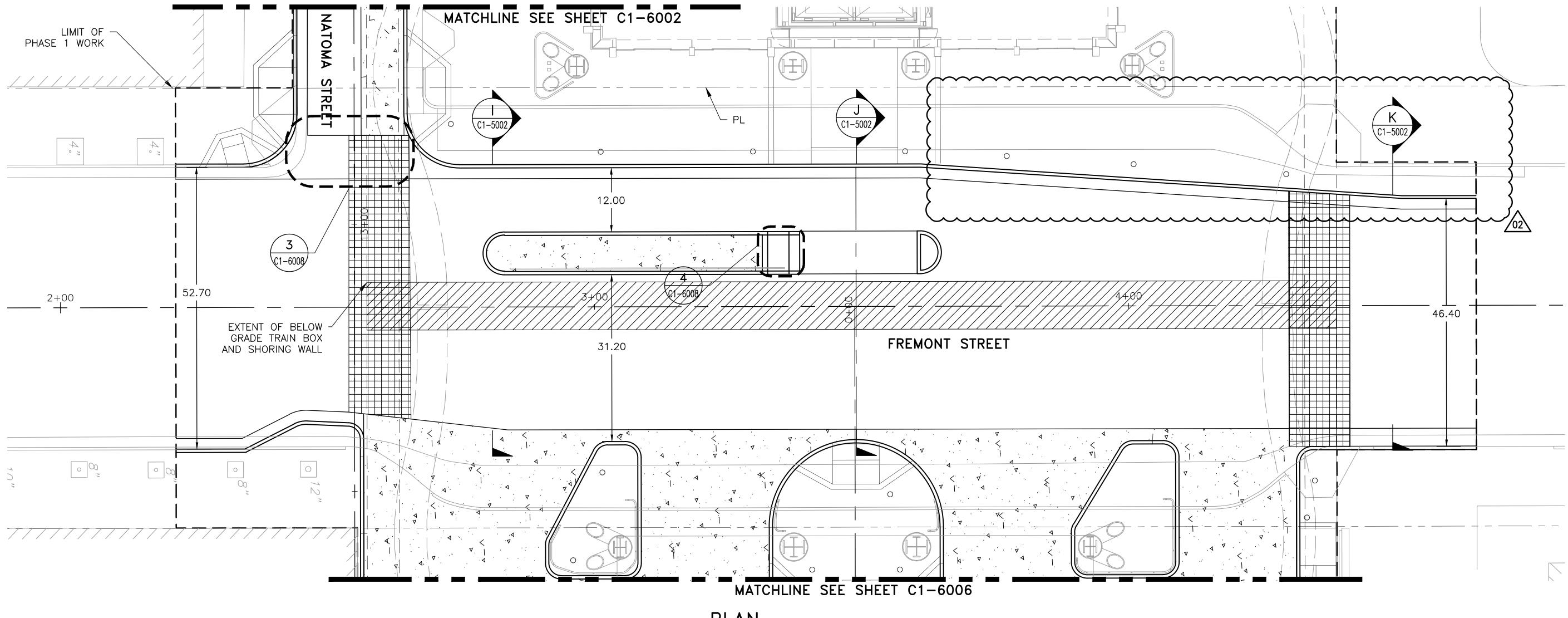
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<u>NOTES</u> FOR GENERAL NOTES SEE SHEET C-0005. FOR LEGEND SEE SHEET C-0010. FOR ABBREVIATIONS SEE SHEET C-0015. F C1-5001 FREMONT STREET SECTION (TYPE) CONCRETE PAVING NATEL PLATE-USED JOINT DETAIL IN TRAIN BOX TRANSITION. PER SECTION 1, C1-6008.



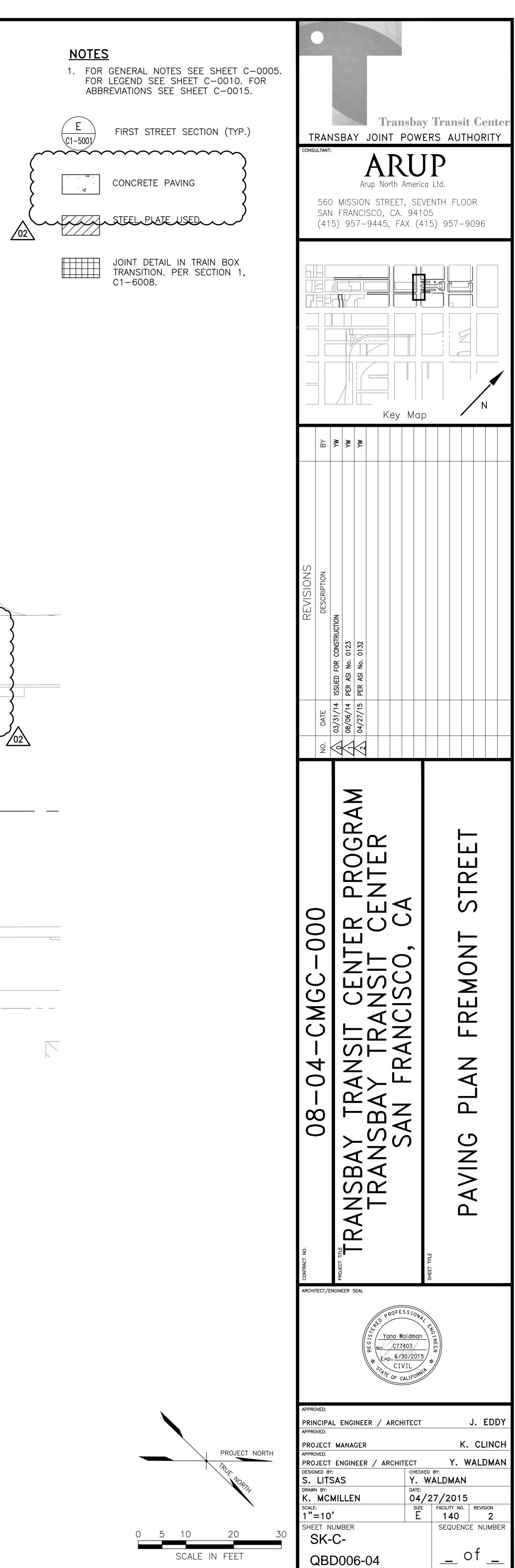


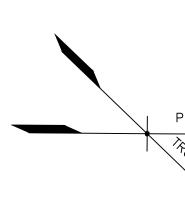




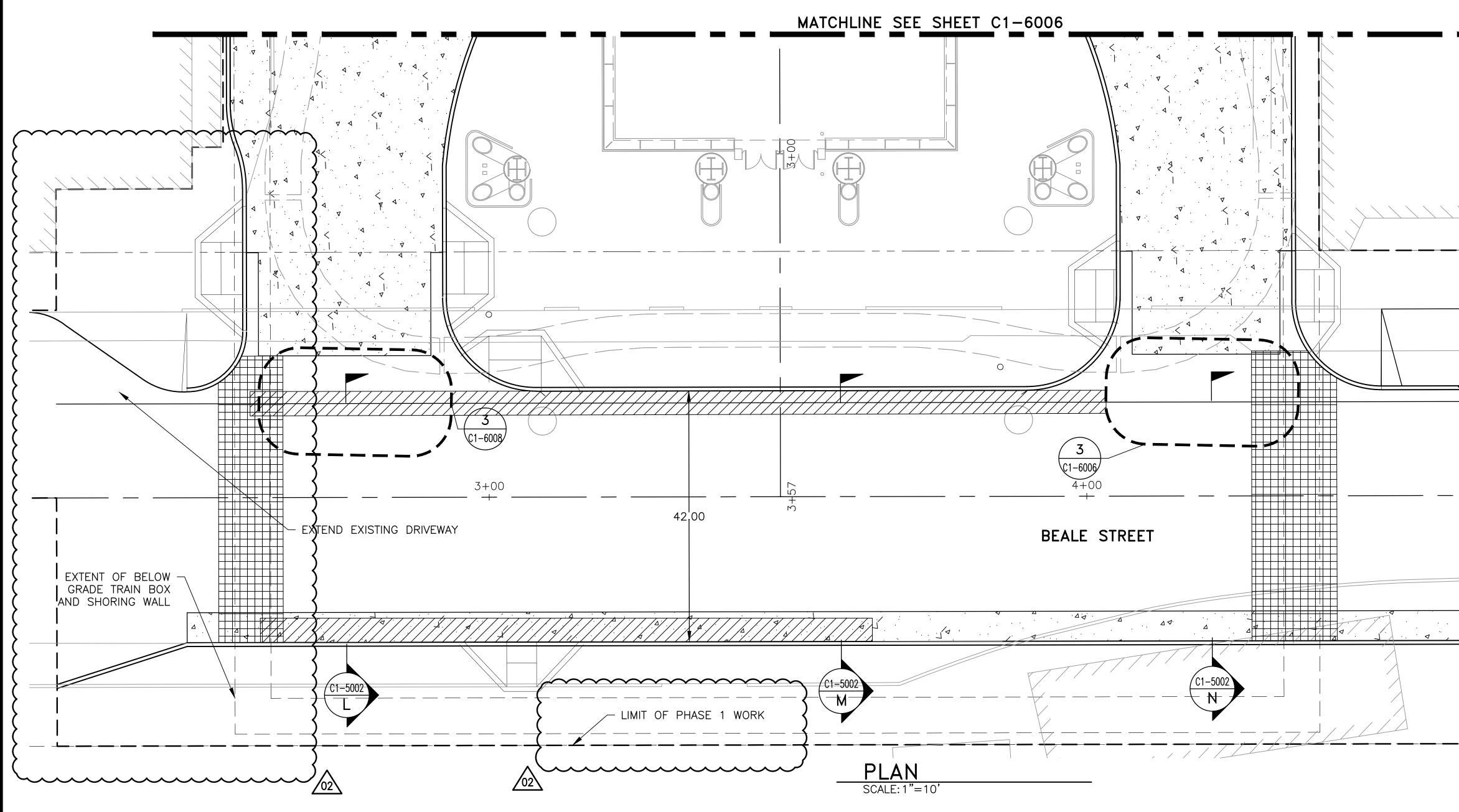
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PLAN SCALE: 1"=10'

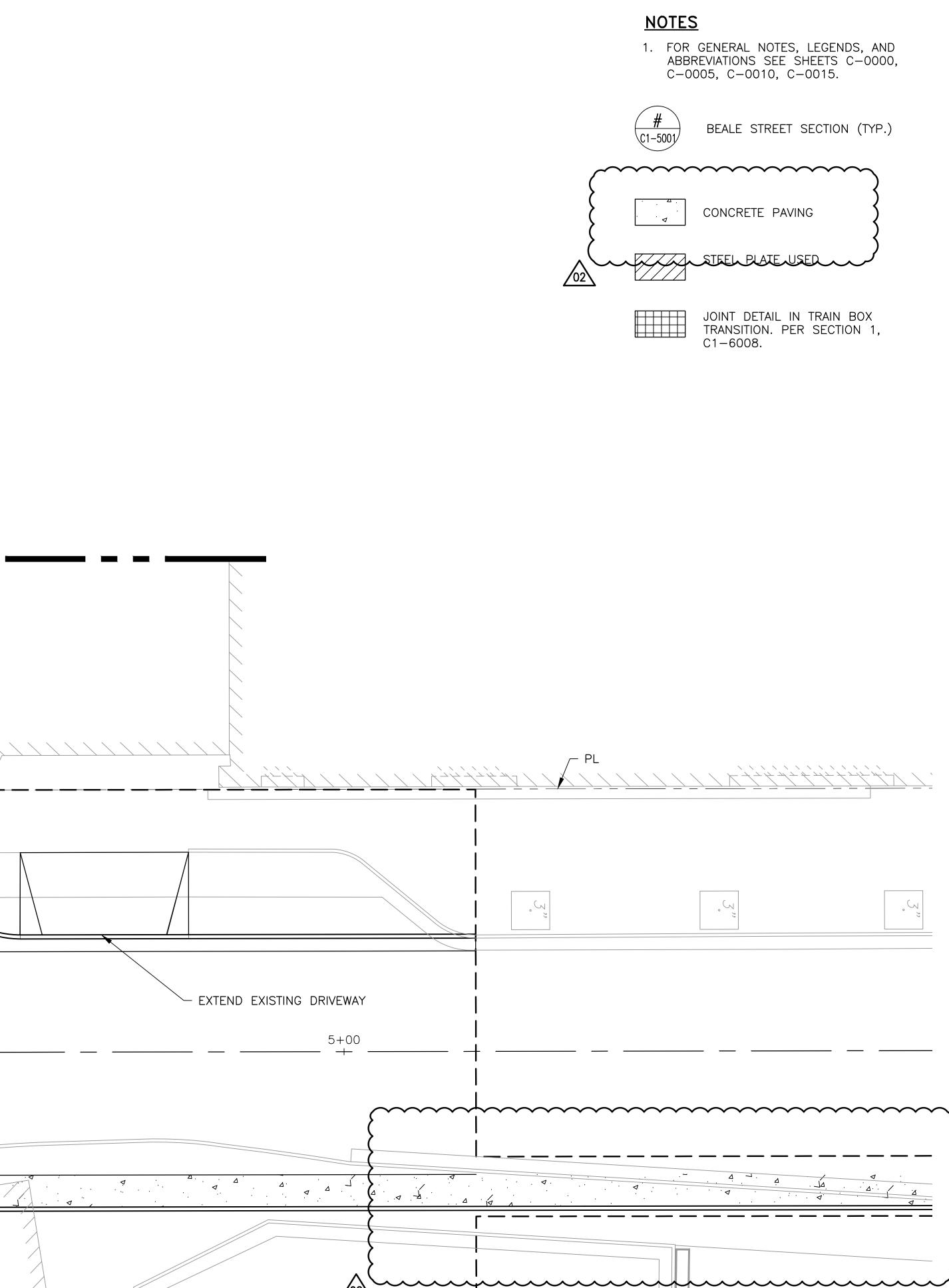




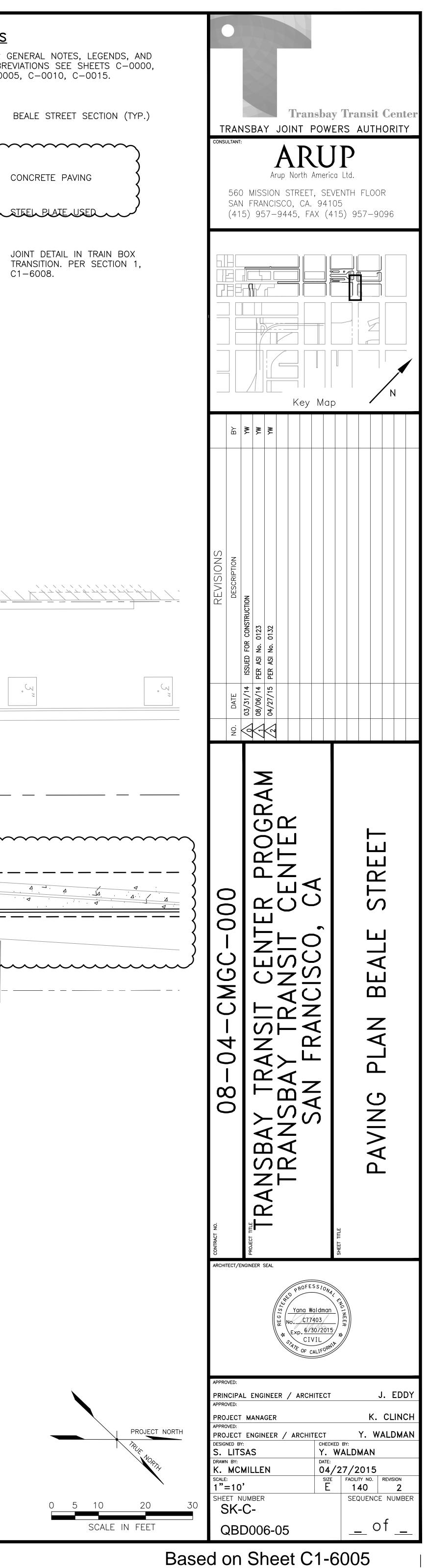




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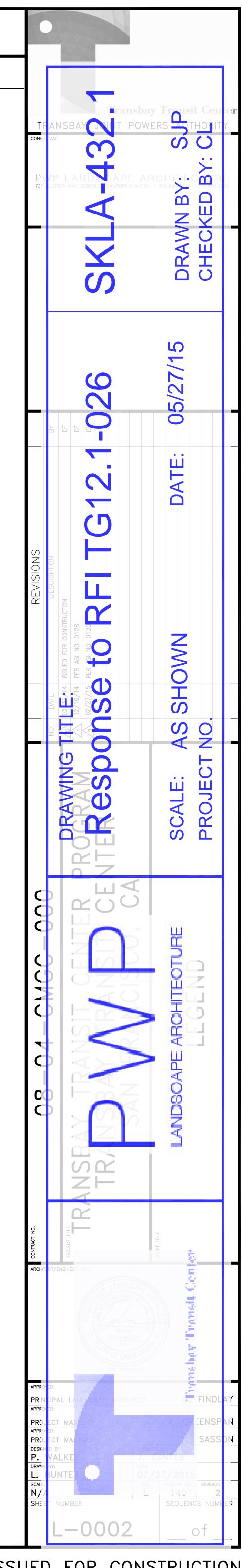
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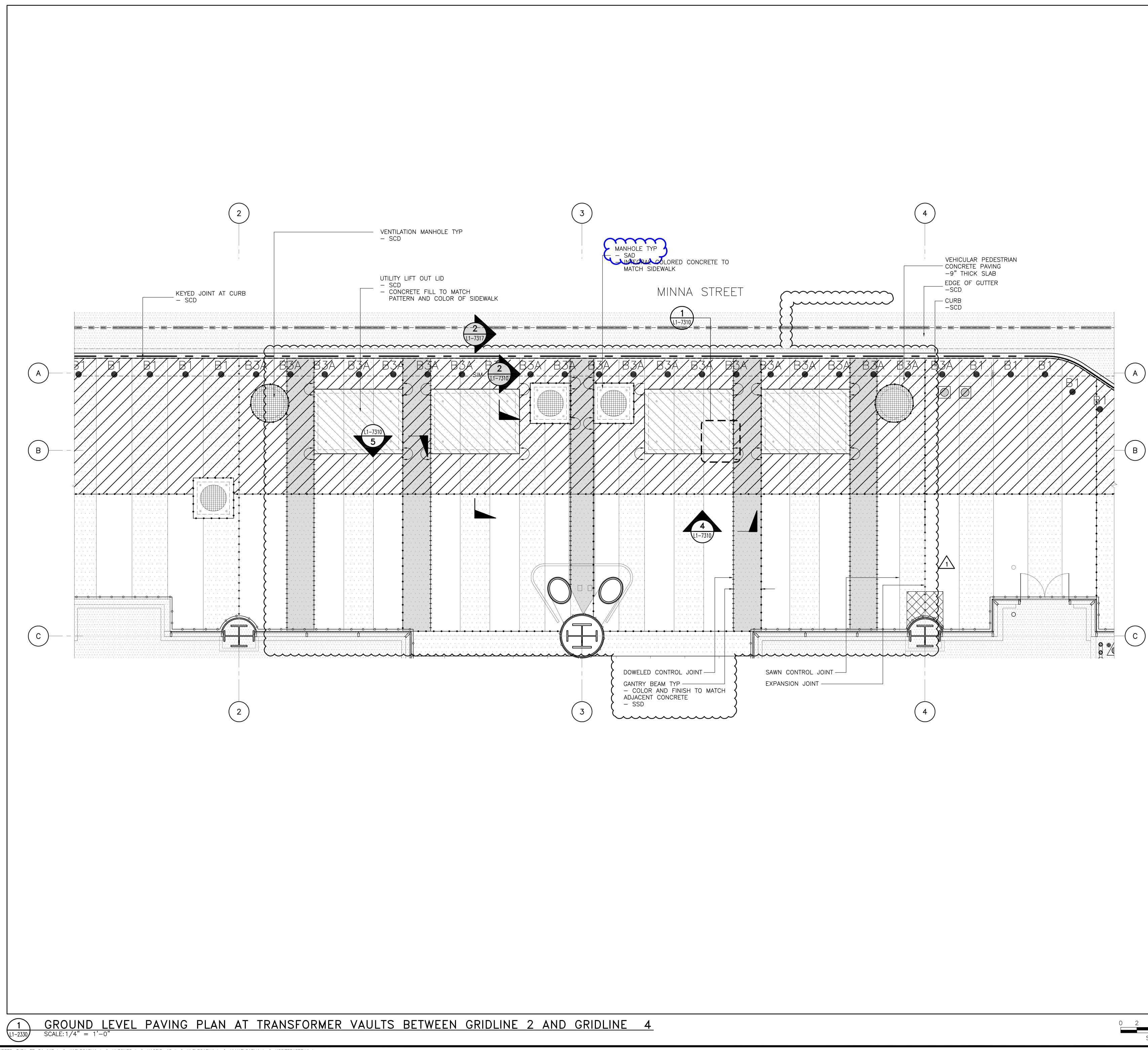
LEGEND	LEGEND	LEGEND	LEGEND
GENERAL ALL SHEETS	GENERAL ALL SHEETS GROUND LEVEL PLANS	GROUND LEVEL MATERIAL PLANS	GROUND LEVEL GRADING AND DRAINAGE PLANS
MATCHLINE MATCHLINE MATCHLINE SECTION/ DETAIL	RADIAL PLANTERS - PRECAST CONCRETE - 15 GAL WOODWARDIA FIMBRIATA - REFER TO 1/L1-7302 FOR PLANTING LAYOUT 2 TRENCH DRAIN IN PAVING	VEHICULAR CONCRETE PAVING ON-STRUCTURE WITH SAW CUT CONCRETE PAVING JOINTS FINISH B FINISH A PEDESTRIAN CONCRETE PAVING ON-STRUCTURE WITH SAW CUT CONCRETE PAVING JOINTS FINISH B FINISH B FINISH A	FS 00.00 PROPOSED FINISHED SURFACE ELEVATION PROPOSED TOP OF CURB ELEVATION
DETAIL NUMBER SHEET NUMBER REVISION CLOUD AND DELTA	MONUMENT BOULDER AT GROUND LEVEL SAW BOTTOM FLAT REFER TO SIGNAGE DRAWINGS FOR TEXT ON BOULDER BIKE RACK	VEHICULAR CONCRETE PAVING ON-GRADE WITH SAW CUT CONCRETE PAVING JOINTS FINISH B FINISH A PEDESTRIAN CONCRETE PAVING ON-GRADE WITH SAW CUT CONCRETE PAVING JOINTS	$\begin{array}{c} \hline & TC & 00.00 \\ \hline & & \\ \hline \hline & & \\ \hline \hline & & \\ \hline \hline \\ \hline & & \\ \hline \hline \\ \hline \hline & & \\ \hline \hline \hline \\ \hline \hline \hline \hline \\ \hline \hline$
ENLARGEMENT	B1 B1 BOLLARD TYPE 1 - STATIONARY BOLLARD TYPE 1A - STATIONARY - STATIONARY - SPECIAL FOOTING AT CURB RAMP	FINISH A AGGREGATE MULCH	RIM 00.00 PROPOSED RIM ELEVATION TDHP 00.00 TRENCH DRAIN HIGH POINT ELEVATION
DESIGN AND CONSTRUCTION WORK OUTSIDE OF LANDSCAPE SCOPE -SAD -SCD BREAKLINE BREAKLINE	B1B BOLLARD TYPE 1B - STATIONARY - SPECIAL FOOTING AT MUNI OCS POLE B1C BOLLARD TYPE 1C - STATIONARY - SPECIAL FOOTING AT CURB RAMP AND MUNI OCS POLE B2 B2 - STATIONARY - STATIONARY - SPECIAL FOOTING AT CURB RAMP AND MUNI OCS POLE	CONCRETE INFILL IN PG&E UTILITY VAULT LIFT OUT LID WITH DECORATIVE CONCRETE PAVING JOINTS FINISH A FINISH B	TDLP 00.00 TRENCH DRAIN LOW POINT ELEVATION INV 00.00 INVERT ELEVATION PIPE DIAMETER
SLOPE RATIO	B2A BOLLARD TYPE 2A - STATIONARY - SPECIAL FOOTING AT CURB RAMP BOLLARD TYPE 2B - STATIONARY - STATIONARY - SPECIAL FOOTING AT MUNI OCS POLE	SPECIALTY CONCRETE PAVING AT SHAW ALLEY SPECIALTY CONCRETE PAVING AT SHAW ALLEY CONCRETE PAVING AT AREAS OF INSUFFICIENT DEPTH TO ACCOMMODATE FULL PAVING PROFILE OMIT PROTECTION SLAB AND REPLACE WITH STEEL PLATE REFER TO ARCHITECTURE DRAWINGS FOR TRAIN BOX LID, SIMILAR TO 2/A1-8710	GRADE BREAK GRADE BREAK I.14% FINISHED SURFACE SLOPE PERCENTAGE FLUSH
	B2C - STATIONARY - SPECIAL FOOTING AT CURB RAMP AND MUNI OCS POLE B3 BOLLARD TYPE 3 - REMOVABLE BOLLARD TYPE 3A - REMOVABLE - SPECIAL FOOTING AT UTILITY VAULTS	- REFER TO ARCHITECTURE DRAWINGS FOR TRAIN BOX LID, SIMILAR TO 2/A1-8710 CRACK CONTROL JOINT EXPANSION JOINT	AREA DRAIN IN PAVING AREA DRAIN IN PLANTING
AREA OF ASSISTED RESCUE PEDESTRIAN PUSH BUTTON -SAD	B4 = BOLLARD TYPE 4 $- STATIONARY = BOL-4$ $BOL = -4$ $- SAD$ $BOL = 5$ $- SAD$	KEYED JOINT SAW CUT CRACK CONTROL JOINT SAW CUT EXPANSION JOINT	CATCH BASIN WITH AREA DRAIN GRATE
FIRE DEPARTMENT CONNECTION - SPD LIMIT OF WORK LINE CURB RAMP	−5 Image: Bollard = Retractable = SAD Image: Signage Pylon Image: Hose Bib	EXPANSION JOINT - NON DOWEL CONT C	CATCH BASIN WITH CLEAN OUT SOLID COVER CATCH BASIN WITH SAND FILTER SOLID PIPE SOLID PIPE
Build adopted by MUNI OCS POLE -SAD	 SPD RETRACTABLE DISPENSER SED IN-GRADE FLUSH UPLIGHT SLD, SED 	AND 2" OF MULCH	PERFORATED PIPE SOLID PIPE SOLD PIPE SCD
rstul Moy E6, 2015 – 507pm ZNPCPNBRINFIERM Silvetthee/NKIA-438 HEI T01E1-E66 IA	FIRE DEPARTMENT CONNECTION - SPD FRESH AIR INLET FRESH AIR INLET -SED FIRE ALARM REMOTE ANNUCIATOR -SED STREET SIGNAGE -SCD	ILLUMINATED PAVER AT SHAW ALLEY METAL CONCRETE CLOSURE WALL AT STAIR 201 -SAD	POINT OF CONNECTION - SPD POINT OF CONNECTION - SCD

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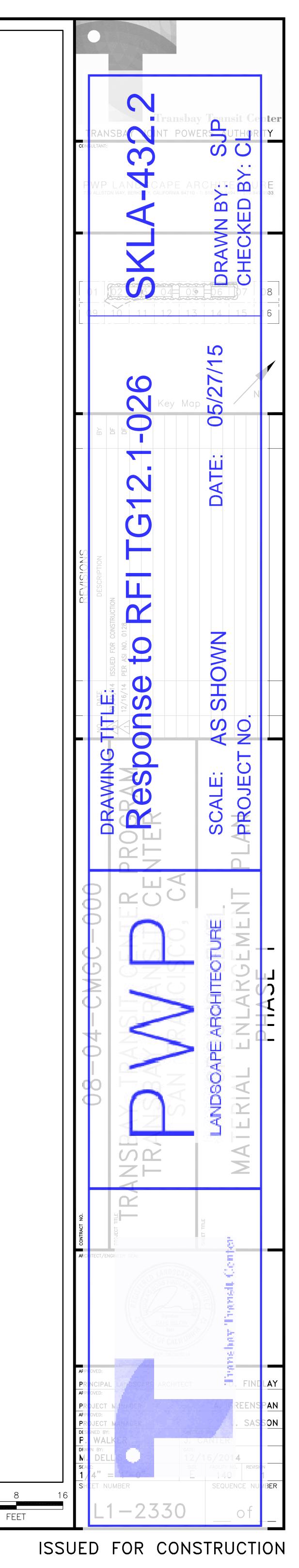


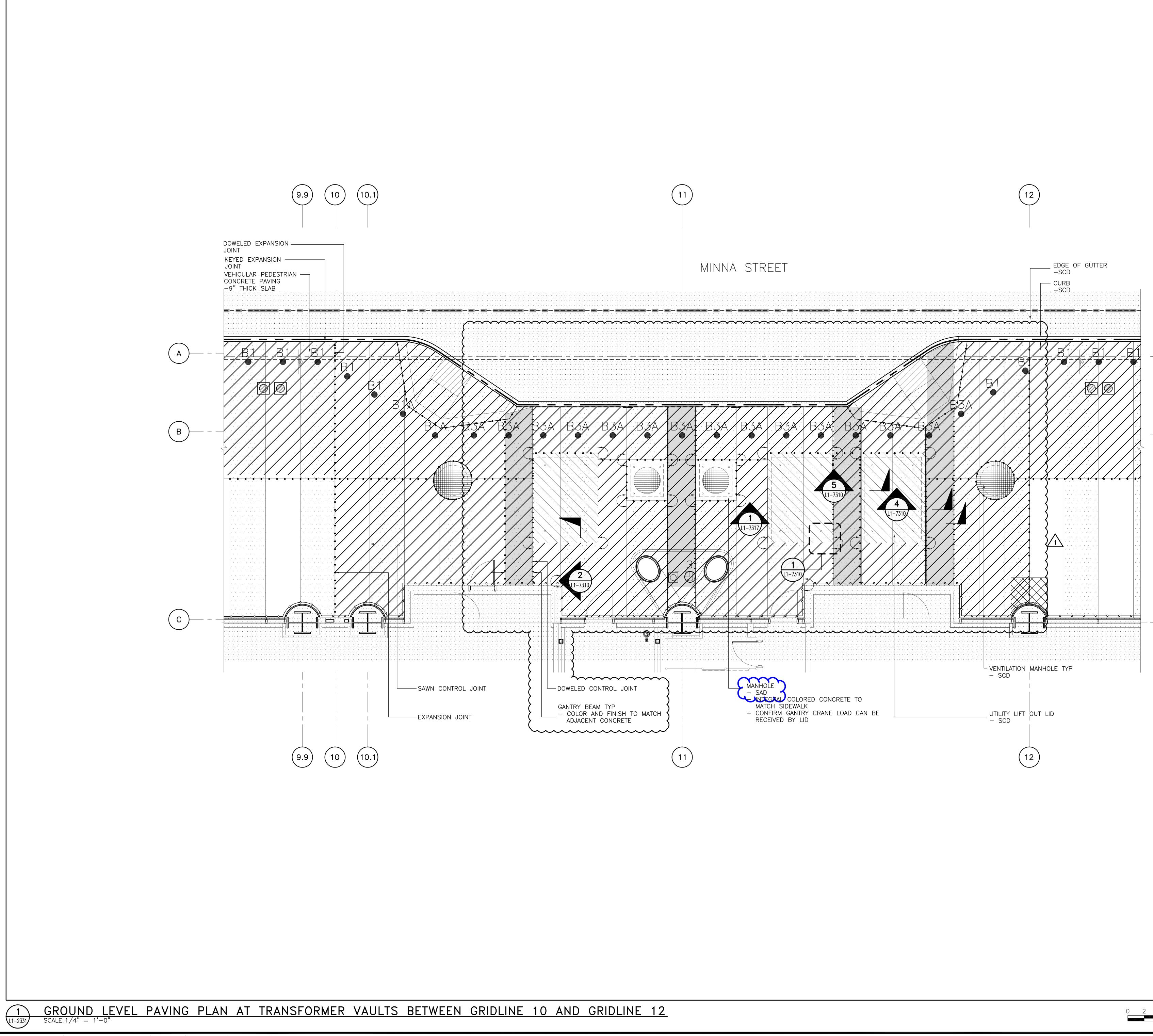
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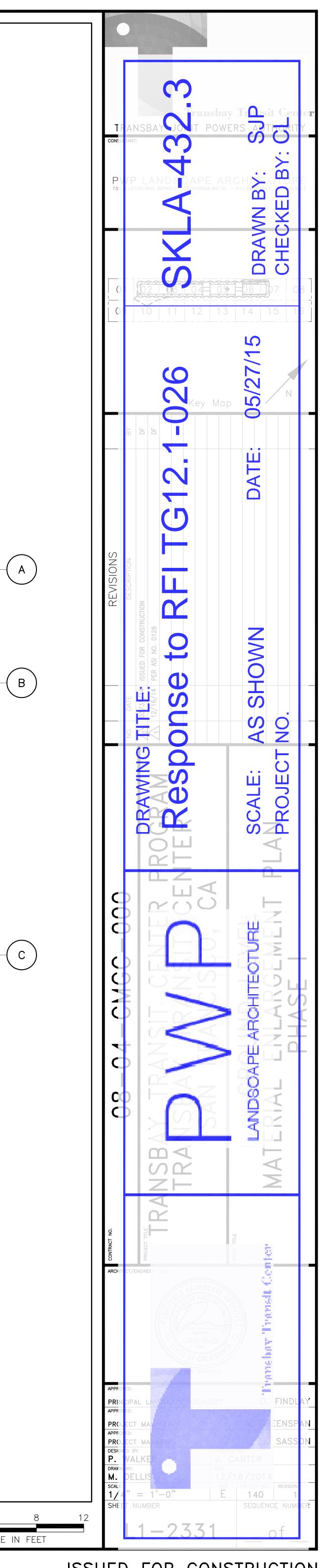
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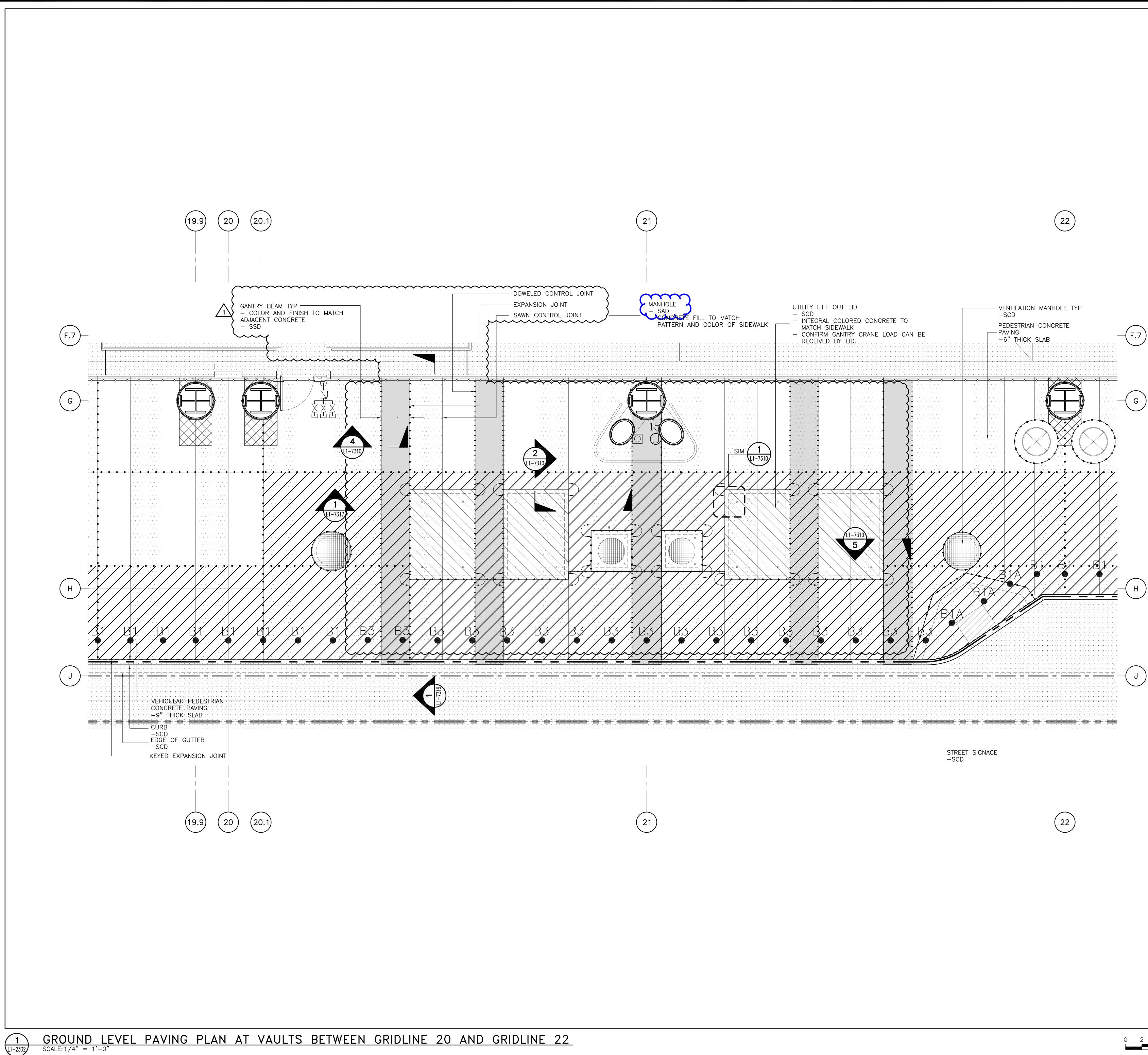




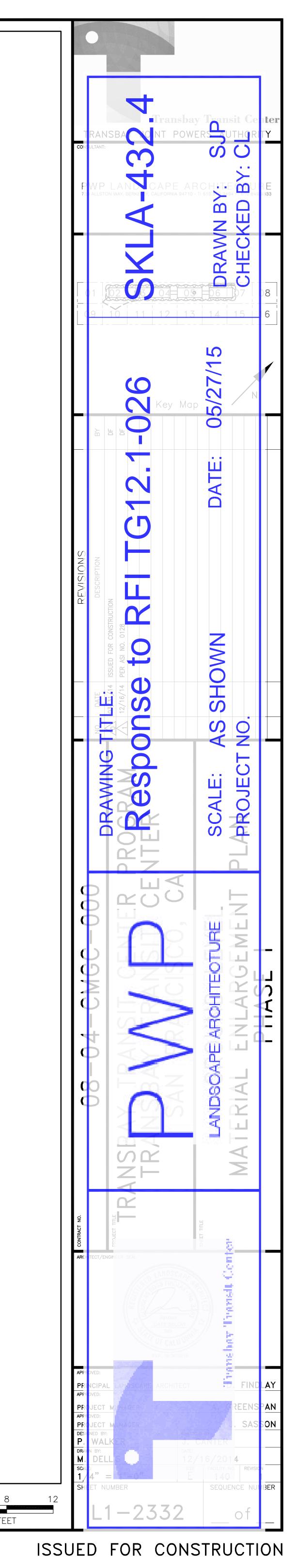
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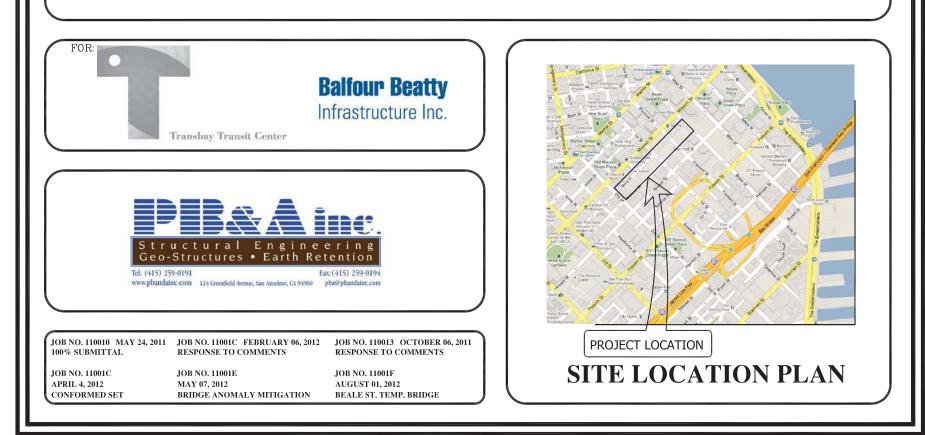
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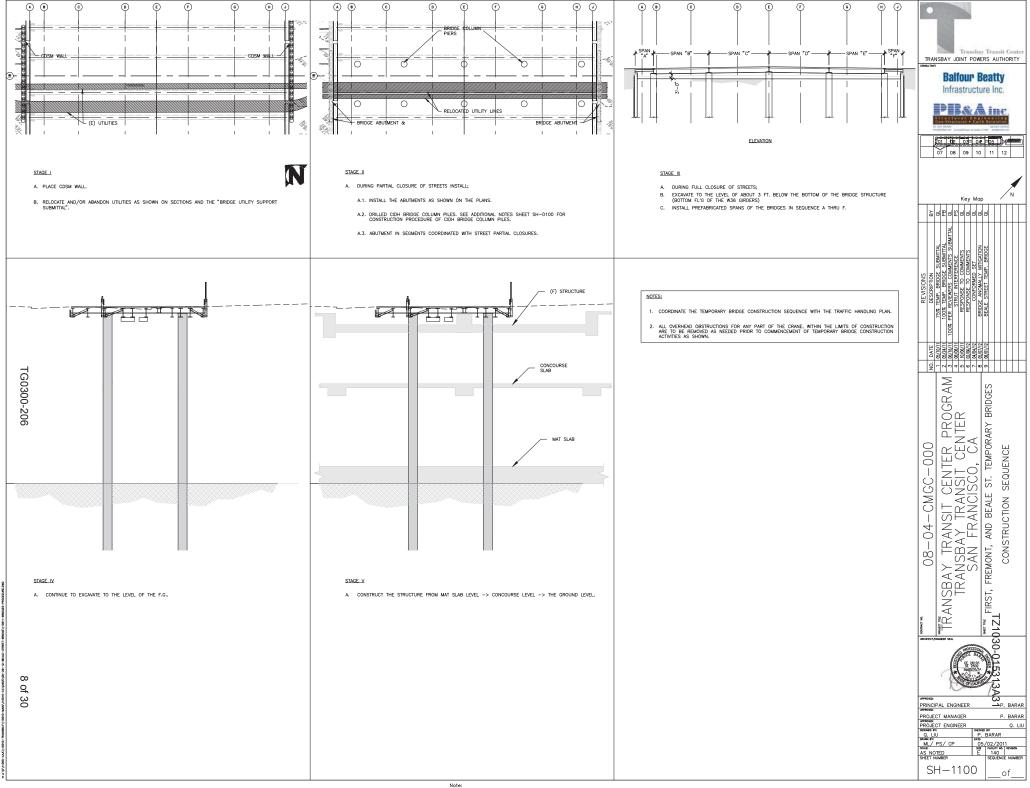
FIRST, FREMONT AND BEALE STREET TEMPORARY BRIDGE STRUCTURAL DESIGN TRANSBAY TRANSIT CENTER PROGRAM TRANSBAY TRANSIT CENTER, SAN FRANCISCO, CA

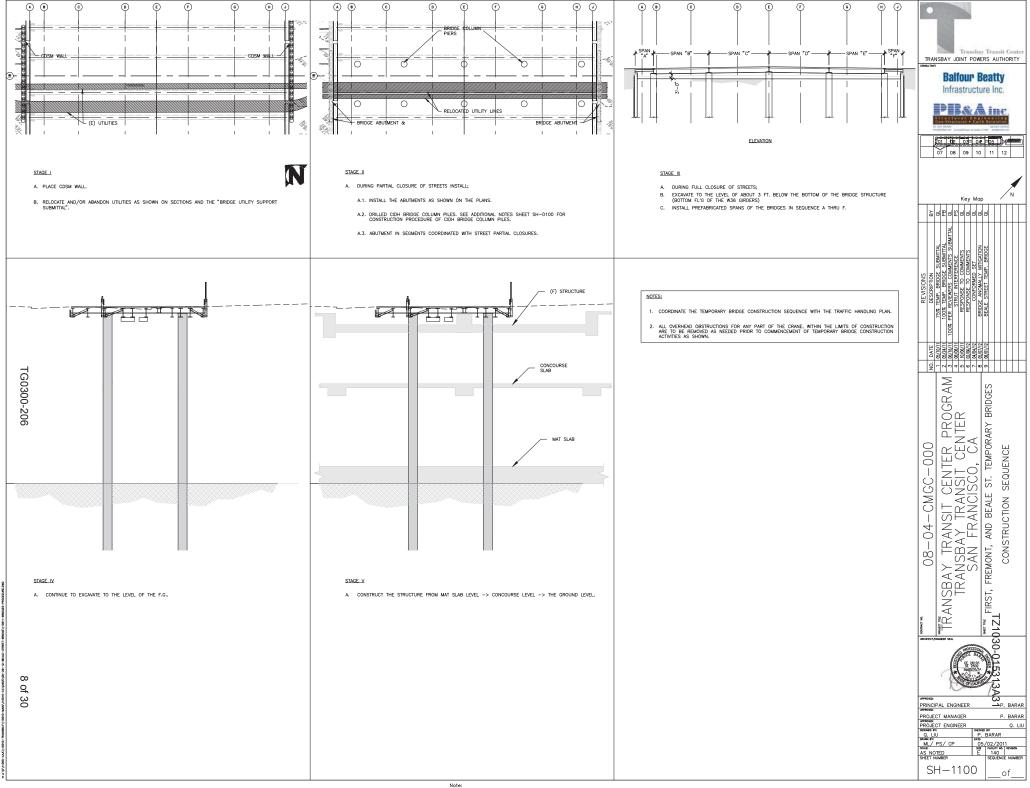


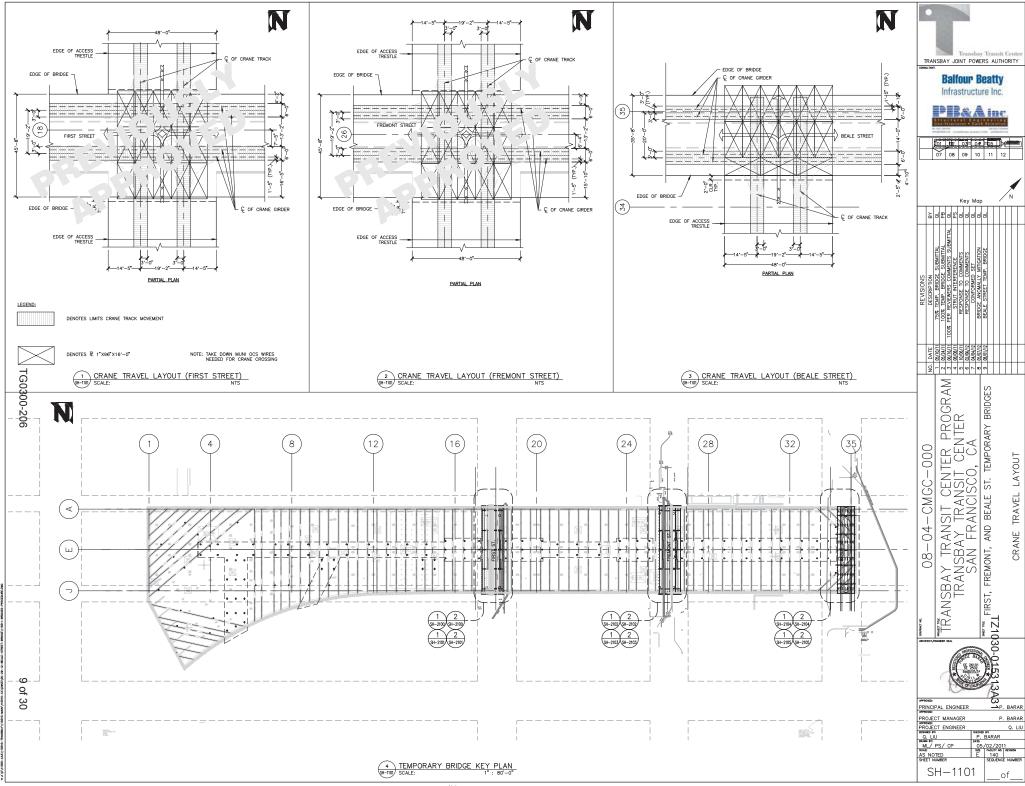
GENERAL NOTES	GENERAL NOTES	GENERAL NOTES	GENERAL NOTES	•
ART 1 - GENERAL	C.3. PLACE REINFORCEMENT CASE IN THE HOLE. USE RIGGING TO SECURE REINFORCEMENT CAGE TO CASING DURING CONCRETE PLACEMENT.	E.3. METAL STUD & TRACK: ASTM A653, MIN. G40 COATING		100
 SCOPE OF WORK A. IN ORDER TO ALLOW CONTINUATION OF TRAFFIC ACROSS THE CONSTRUCTION SITE EXCAVATION AT FIRST, FREMONT, AND BEALE STREETS FOR A TOTAL OF THREE (3) BRIDGES. BRIDGES AT 	C.4. PLACE CONCRETE BY TREMIE METHOD FROM TIP ELEVATION TO THE TRANSITION CONSTRUCTION JOINT AS INDICATED ON THE PLANS. AS THE CONCRETE IS PLACED SLURRY IS TO BE PUMPED BACK INTO SLURRY TAMKS	F. PLAIN ELASTOMERIC BEARING F.1. THE MATERIAL SPECIFICATIONS SHALL MEET ALL REQUIREMENTS OF ASHTO M251.		Transbay Transit Cen TRANSBAY JOINT POWERS AUTHORIT
EACH CROSSING SHALL PROVIDE A CONNECTION ACROSS THE CONSTRUCTION EXCAVATION, FROVUNING LANES ON BRIDGES TO PROVIDE A PATH FOR VEHICLES AND PEDESTRIANS ACROSS THE CONSTRUCTION SITE.	C.5. THE RIGGING IS TO REMAIN IN PLACE UNTIL THE CONCRETE SETS.	F.2. PLAIN ELASTOMERIC BEARINGS SHALL CONFORM TO SECTION $51-1.12H(1)$ OF CALTRANS STANDARD SPECIFICATIONS.		Balfour Beatty
B. RELATED WORK NOT INCLUDED IN THIS PACKAGES:	C.7. INSTALL THE CONNECTION ELEMENTS AT THE TOP OF BRIDGE COLUMN PILE			Infrastructure Inc.
B.1. CONCRETE BASEMENT WALL,	AS INDICATED ON THE PLANS. C.8. PREPARE THE CONSTRUCTION JOINT.	SURFACE OF THE PTFE AND THEIR DEPTH SHALL BE NOT LESS THAN 0.08" AND NOT MORE THAN HALF THE THICKNESS OF THE PTFE. THE		PIR&A inc.
B.2. SHORING OR BRACING FOR EXISTING BUILDINGS, B.3. TRESTLES,	C.9. POUR CONCRETE TO THE DESIGNED TOP OF PILE ELEVATION.	RESERVOIRS SHALL BE UNIFORMLY DISTRIBUTED OVER THE SURFACE AREA AND SHALL COVER MORE THAN 20% BUT LESS THAN 30% OF IT. LUBRICANT SHALL BE SILICONE GREASE WHICH SATISFIES MILITARY SPECIFICATION MIL-S-B660.		STUDIES CONTRACTOR
B.4. STRUT REMOVAL AND REBRACING	2.2 MATERIALS	G. POLYETHYLENE VAPOR BARRIER: ASTM E1745 CLASSES A, B, & C		(partmanage part falling
B.5. PLANNED CONSTRUCTION, OR EXCAVATION OTHER THAN NOTED ABOVE.	A. CONCRETE AT DRILLED SHAFTS AND ABUTMENTS	H. PLYWOOD: APA RATED CD-X, STRUC I		Constantiation (
B.6. BRIDGE TRAFFIC: SIGNAL & SIGNAGE	A.1. PORTLAND CEMENT ASTM C-150 TYPE "I" OR TYPE "IA"	PART 3 - Q/A PROGRAM		
B.7. BRIDGE LIGHTING B.8. BRIDGE MUNI OCS	A.2.NORMAL WEIGHT CONCRETE = 3,500 PSI AT 28 DAYS, PROPORTIONED WITH A MAXIMUM WATER CEMENT RATIO, W/C=0.50. ALL CONCRETE SHALL BE BATCHED, DELIVERED, AND PLACED IN ACCORDANCE WITH	3.1. SPECIAL INSPECTION, INSPECTION PROCEDURES AND SITE OBSERVATION		
B.9. UTILITY SUPPORT DETAILS	ACI 301 AND ASTM C94.	A. SPECIAL INSPECTIONS		
B.10. TRAFFIC CONTROL PLANS	A.3.SUBMIT MIX DESIGN PREPARED BY A PROFESSIONAL ENGINEER REGISTERED IN THE STATE OF CALIFORNIA FOR ENGINEER'S APPROVAL.	A.1. THE SPECIAL INSPECTORS MUST BE CERTIFIED BY TJPA TO PERFORM		Key Map
C. RELATED DOCUMENT:	B. STRUCTURAL LIGHTWEIGHT CONCRETE AT PEDESTRIAN WALK	THE TYPE OF INSPECTION SPECIFIED.		22222222222
C.1. "FINAL GEOTECHNICAL DATA REPORT" PREPARED BY ARUP, DATED FEBRUARY 2010	B.1.PORTLAND CEMENT ASTM C-150 TYPE "I" OR TYPE "IA	A.2.SPECIAL INSPECTIONS SHALL BE PROVIDED BY THE TUPA FOR PERMANENT AND TEMPORARY WORK ACCORDING TO THE CALIFORNIA BUILDING CODE (CBC) CHAPTER 17 INCLUDING THE SAN FRANCISCO		7
C.2. CONTRACT 08-04-CMGC-000 DWG'S, REV. 0 DATED 12-10-10. C.3. STRUCTURAL COMBINED.DWG - DATED 9/01/10	B.2.AGGREGATE: EXPANDED SHALE, CLAY, OR SLATE (ESCS) LIGHTWEIGHT AGGREGATE PRODUCED BY THE ROTARY KILN METHOD SHALL MEET ASTM C330.	AMENDMENTS, AND THE PROJECT SPECIFICATIONS. THE SPECIAL INSPECTOR SHALL OBSERVE THE WORK FOR CONFORMANCE WITH THE CONSTRUCTION DOCUMENTS, THE SPECIAL INSPECTOR SHALL SEND REPORTS TO THE OWNER, ARCHITECT, ENGINEER, BUILDING OFFICIAL		SIONS aptions Benoes Submittal Benoes Submittal Benoes Submittal Interference En comments En comments En comments En comments En comments En comments En comments En teme Benoes
C.4. XSURVEY.DWG - DATED 7/27/2010	B.3.GROUND GRANULATED BLAST-FURNACE SLAG SHALL MEET ASTM C989.	AND THE CONTRACTOR IN A TIMELY MANNER. ALL DISCREPANCIES SHALL BE BROUGHT TO THE ATTENTION OF THE CONTRACTOR FOR		EENTS GATIO GATIO
C.5. XTRAF-SIGN.DWG - DATED 7/27/2010		CORRECTION, WHEN WORK IS DONE TO THE SATISFACTION OF THE INSPECTOR, THEN THE SPECIAL INSPECTOR SHALL SUBMIT A FINAL		MITIC SUE
C.6. RESPONSE TO RFI T-0073.1 DATED APR. 14, 2011: BSE - REQUEST FOR RESPONSE SPECTRA	B.4.MATERIALS SHALL BE PROPORTIONED TO PRODUCE CONCRETE WITH A MINIMUM COMPRESSIVE STRENGTH = 3,000 PSI IN 28 DAYS.	SIGNED REPORT STATING THAT, TO THE BEST OF THEIR KNOWLEDGE, THE WORK WAS COMPETED IN CONFORMANCE WITH THE PLANS, SPECIFICATIONS, AND THE APPLICABLE WORKMANSHIP PROVISIONS OF	Mean Horizontal Equal Hazard Spectra East End of Train Box (475 Return Period) FINAL STACE - TEMPORARY BRIDGE DESIGN	PTION SS COOL
C.7. RESPONSE TO RFI T-0209.3 DATED SEP. 28, 2011: BSE - ABUTMENT BEARING ON COSM WALL - FOLLOW-UP	B.5.DENSITY: MATERIALS SHALL BE PROPORTIONED TO PRODUCE CONCRETE WITH A CALCULATED EQUILIBRIUM DENSITY OF 110 PCF ± 3 PCF AS DETERMINED BY ASTM C567-00, SECTION 9.2	THE CBC.	1.2	STREE STREE STREE STREE
C.8. RESPONSE TO RFI T-0219 DATED SEP. 14, 2011: BSE - ABUTMENTS AT TEMPORARY BRIDGES	C. STRUCTURAL STEEL SHALL BE NEW EXCEPT THE W14 GIRDERS AND CAP BEAM FOR THE END SPANS AND IT SHALL MEET OR EXCEEDS THE	A.3.ITEMS REQUIRING SPECIAL INSPECTIONS; A.3.1. INSPECTION OF MILL CERTIFICATES OR LAB RESULTS FOR ALL		REVISION PESORPIO DESORPIO DESORPIO DESORPIO PER REVERSES STRUT INTER RESPONSE TO RESPONSE TO RESPONSE TO RESPONSE TO RESPONSE TO RESPONSE TO RESPONSE TO
C.9. RESPONSE TO RFI T-0222 DATED SEP. 01, 2011: BSE - TEMPORARY BRIDGE PIER LOCATIONS	FOLLOWING REQUIREMENTS	COMPONENTS OF THE TEMPORARY BRIDGES.	<u><u><u></u></u><u></u><u></u><u></u><u></u><u></u><u></u><u></u><u></u><u></u><u></u><u></u><u></u><u></u><u></u><u></u><u></u><u></u></u>	800
C.10. RESPONSE TO RFI T-0223 DATED SEP. 27, 2011: BSE - TEMPORARY	C.1.STEEL SHAPES: ASTM A992, GRADE 50 Fy = 50 KSI	A.3.2. AWS PREQUALIFIED WELDING PROCEDURES, OR DOCUMENTATION OF QUALIFICATION PERFORMED FOR PROJECT SPECIFIC WELDING PROCEDURES IN ACCORDANCE WITH AWS	5 0.6	-
BRIDGE PEDESTRIAN BARRIER HEIGHT	C.2.STEEL REINFORCEMENT: ASTM A615, GRADE 60 Fy = 60 KSI	D1.5 REQUIREMENTS.	¥ 0.4	DATE 0.411/0 0.6/10/11 0.06/11 0.06/11 0.06/12 0.0/02 0.0/12 0.0/12
C.11. RESPONSE TO RFI T-0228 DATED SEP. 27, 2011: BSE - 6-INCH SIDEWALK AT TEMPORARY BRIDGES	C.3.STEEL PLATES: ASTM A572, GRADE 50 Fy = 50 KSI	A.3.3. TESTING OF STRUCTURAL WELDS	NO2	0 - 0 0 + 0 0 - 0 0
C.12. PROJECT SPECIFICATION ADDENDUM NO. 3 SECTION 01 53 13 - TEMPORARY BRIDGES DATED SEPTEMBER 23, 2010	C.4.STEEL ANGLES: ASTM A36, GRADE 36 Fy= 36 KSI, U.N.O. C.5.STEEL PIPE SECTIONS FOR CONNECTION BETWEEN CIDH PILE AND CAP	A.3.4. INSTALLATION OF ALL A490-X BOLTS FASTENERS (UNLESS "TENSION CONTROLS" BOLTS ARE USED) INCLUDING INCREDIDATION AND ACCEPTINCE OF DOLL USED DIOD		
C.13. CAD FILES RECEIVED IN RESPONSE TO RFI T-0035.1 DATED MAR. 23, 2011: BSE - REQUEST FOR STRUCTURE SECTION DRAWINGS	BEAM: ANSI B36.1/ASTM A53, FY = 35 KSI.	INSPECTION AND ACCEPTANCE OF BOLT HOLES PRIOR TO INSTALLATION OF FASTENERS. INSPECTION IS TO VERIFY PROPER PREPARATION OF HOLES, SPECIALLY NO OVERSIZED	PERIOD (SEC)	AM
CODES AND STANDARDS	C.6.STEEL BOLTS: ASTM A490-X BOLTS, MIN. TENSILE STRENGTH = 150 KSI	HOLES OR FLAME CUTTING HAS BEEN USED TO INCREASE TOLERANCES TO ACHIEVE FIELD FIT.		
STRUCTURAL PERFORMANCE: BRIDGES SHALL BE PROVIDED MEETING DESIGN LOADING CRITERIA COMPLIANT WITH THE	C.7.STANDARD MILD STEEL STUDS MANUFACTURED BY NELSON CONFORM TO ASTM A108 SPECIFICATIONS FROM 1010 THROUGH 1020 MILD	A.3.5. VERIFICATION THAT EXCAVATIONS ARE EXTENDED TO THE PROPER DEPTH.		
CURRENT AMERICAN ASSOCIATION OF STATE HIGHWAY AND TRANSPORTATION OFFICIALS (AASHTO) LRFD DESIGN	STEEL.	A.3.6. PLACEMENT OF CONCRETE		C PR ENTE CA ORARY
SPECIFICATIONS, 4TH EDITION, AND CALTRANS AMENDMENTS. A.1. LIMIT STATE DESIGN	C.8.WELDED WIRE REINFORCEMENT: ASTM A185, GRADE 65	A.4. CONTRACTOR SHALL INSPECT BRIDGE PAVEMENT FOLLOWING USAGE		PORAN H
A.1.1. STRENGTH I - DC+DW+HL-93 DESIGN VEHICLE W/ 33% IM	C.9.STRUCTURAL METAL DECK: ASTM A653 GR. 50, 4 ∦ DEEP, MAX ALLOWABLE CLEAR SPAN BETWEEN GIRDER FLANGES FOR HL-93 &	FOR TRAVEL BY CONTRACTOR EQUIPMENT. ALL DAMAGE OR SURFACE IRREGULARITIES RESULTING FROM SUCH USAGE SHALL BE IMMEDIATELY		¥ ○∐ S
A.1.2. STRENGTH II - DC+DW+CALIFORNIA PERMIT TRUCK P15 W/ 25% IM	CALIFORNIA PERMIT TRUCK P15 LOADING OF 57 INCHES AND FOR MANITOWOC 999 SERIE 2 CRANE OF 26 INCHES DESIGNED IN	CORRECTED.		
A.1.3. SERVICEABILITY AND FATIGUE CHECKS	ACCORDANCE WITH SECTION 1.2 CODES AND STANDARDS. THE MANUFACTURER IS TO PROVIDE THE CERTIFICATION FOR THE REFERENCED LOADING.	A.5. APPROACH TRANSITIONS FROM ROAD TO BRIDGES SHALL BE INSPECTED FREQUENTLY FOR SETTLEMENT. REPAIR WITH HOT ASPHALT CONCRETE PAVEMENT AS NECESSARY.		
A.1.4. CONSTRUCTION LOADS W/ MANITOWOC-999 SERIES 2 CRANE LOADING WITHOUT ANY HOOK LOAD (WITHIN DESIGNATED AREAS ONLY)	C.10.ALL WELDS SHALL BE MADE USING LOW-HYDROGEN ELECTRODES WITH MINIMUM TENSILE STREINGTH = 70 KSI. SEE SPECIFICATIONS FOR FURTHER REQUIREMENTS.	A.G. CONTRACTOR SHALL ON A MONTHLY BASIS INSPECT AND REPAIR SURFACE IRREGULARITIES IN BRIDGE PAVEMENT OCCURRING IN VEHICLE AND PEDESTRIAN AREAS.		
A.2. SEISMIC DESIGN	C.11.IN COMPLIANCE TO WELDING PROCEDURES AND SPECIFICATIONS' THE	A.7.SPECIAL INSPECTION ITEMS OF METAL DECK	DRAWING INDEX	
A.2.1. BASED ON CALIFORNIA SEISMIC DESIGN CRITERIA (SDC), VERSION 1.6, NOVEMBER 2010	FOLLOWING PROCEDURES AND/OR PREPARATIONS ARE TO BE FOLLOWING PROCEDURES AND/OR PREPARATIONS ARE TO BE	PERIODIC INSPECTIONS SHALL BE PERFORMED TO CONFIRM THE PERFORMANCE OF THE METAL DECK WELDS WITH FATIGLE LOADING	SH-0100 GENERAL NOTES AND DRAWING INDEX	AND ANS
A.2.2. DISPLACEMENT DEMANDS ARE CALCULATED BASED ON STRUCTURAL FUNDAMENTAL PERIOD AND DESIGN	C.11.1. JOINT PREPARATIONS AND WELDING PROCEDURES INCLUDING	THE INSPECTION SHALL CONSIST OF DYE PENETRANT, MAGNETIC PARTICLE OR OTHER NON-DESTRUCTIVE METHOD AS APPROPRIATE FOR THE CONFIGURATION OF THE WELD AND THE DEGREE OF	SH-0100 GENERAL NOTES AND DRAWING INDEX SH-0101 LEGENDS, SYMBOLS AND ABBREVIATIONS	
ACCELERATION RESPONSE SPECTRA.	BUT NOT LIMITED TO: WELDING PROCEDURES, REQUIRED ROOT OPENINGS, ROOT FACE DIMENSIONS, GROOVE ANGLES, BACKING BARS, COPES, SURFACE ROUGHNESS VALUES, AND	PREPARATION, TO CONFIRM THAT FATIGUE CRACKS ARE NOT FORMING. ON THE TWO MOST HEAVILY TRAVELED BRIDGES, FIRST STREET AND	SH-1100 CONSTRUCTION SEQUENCE	
A.2.3. DISPLACEMENT CAPACITY IS BASED ON DUCTILITY OF THE STRUCTURE AND DETERMINED BY PUSH-OVER ANALYSIS IN BOTH TRANSVERSE AND LONGITUDINAL	WELDING TAPERS OF UNEQUAL PARTS.	FREMONT STREET, THREE INSPECTIONS WITHIN THE RIGHT LANE (ASSUMED TO BE THE MOST HEAVILY TRAVELED BY TRUCKS) SHALL	SH-1101 CRANE TRAVEL LAYOUT	
DIRECTION.	C.11.2. SEQUENCING AND PROCEDURES OF WELDING TO MINIMIZE THE EFFECT OF SHRINKAGE, RESIDUAL STRESSES, AND TO	ÈE IDENTIFIED FOR TESTING. THESE LOCATIONS SHALL BE ÁS FOLLOWS:	SH-2100 PARTIAL PLAN FIRST STREET BRIDGE SH-2101 PARTIAL PLAN FIRST STREET BRIDGE	FRE AB
STANDARD SPECIFICATIONS 2006, CALTRANS	MAINTAIN ERECTION TOLERANCES.	1. DIRECTLY OVER A STEEL GIRDER, DIRECTLY OVER THE BENT CAP	SH-2102 PARTIAL PLAN FREMONT STREET BRIDGE	
C. AMERICAN WELDING SOCIETY, AWS D1.5: 2010 'BRIDGE WELDING CODE", 6TH EDITION	D. ASPHALT CONCRETE D.1.ASPHALT CONCRETE SHALL BE TYPE A UNLESS OTHERWISE SPECIFIED	2. DIRECTLY OVER A STEEL GIRDER, CENTERED BETWEEN BENT CAPS	SH-2103 PARTIAL PLAN FREMONT STREET BRIDGE	AN AN
 AMERICAN INSTITUTE OF STEEL CONSTRUCTION (AISC) STEEL DESIGN MANUAL, 13TH EDITION 	BY THE PROJECT SPECIFICATIONS.	 IF THERE IS A LOCALIZED DETERIORATION OF ASPHALT, AT A SITE OF DETERIORATION. IF NO DETERIORATION, AT A RANDOM LOCATION. 	SH-2104 PARTIAL PLAN BEALE STREET BRIDGE SH-2105 PARTIAL PLAN BEALE STREET BRIDGE	ZT
2 - CONSTRUCTION	D.2. ASPHALT BINDER SHALL BE PERFORMANCE GRADE 64-10 PAVING ASPHALT CONFORMING TO SECTION 92, "ASPHALT", OF THE CALTRANS DESCRIPTION OF DESCRIPTION OF DESCRIPTIONO OF D	AT EACH IDENTIFIED LOCATION, AN AREA OF APPROXIMATELY 1-FOOT BY 2-FOOT 6-INCHES OF ASPHALT SHALL BE REMOVED TO EXPOSE	SH-3100 TEMPORARY BRIDGE KEY ELEVATION	
CONSTRUCTION PROCEDURE	STANDARD SPECIFICATIONS UNLESS OTHERWISE SPECIFIED ON THE PROJECT SPECIFICATIONS.	AT LEAST THREE WELDS FOR TESTING. IF FATIGUE CRACKS ARE FOUND, ADDITIONAL TESTING MAY BE NECESSARY TO VERIFY THE EXTENT OF	SH-3101 TRANSVERSE ELEVATION FIRST STREET BRIDGE	
A. CONSTRUCTION PROCEDURE FOR THE CDSM WALLS WILL BE COVERED UNDER OTHER SUBMITTALS.	D.3.THE PERCENTAGE OF AIR VOIDS IN THE MIX DESIGN AT THE TARGET ASPHALT BINDER CONTENT SHALL BE BETWEEN 3% AND 5%.	CRACKING AND POTENTIAL REPAIRS. THE FHA MANDATED TWO YEAR INSPECTION ERFOLIENCY IS APPROPRIATE FOR THIS STRUCTURE	SH-3102 TRANSVERSE ELEVATION FREMONT STREET BRIDGE	-01
B. CONSTRUCTION PROCEDURE FOR INSTALLATION OF TEMPORARY BRIDGES.	D.4. AGGREGATE USED IN ASPHALT CONCRETE SHALL CONFORM TO THE	INCREASE INSPECTION FREQUENCY IF ACP DETERIORATION IS OBSERVED. TJPA AND/OR DESIGNATED REPRESENTATIVE CAN REQUEST INSPECTION IF THERE IS EVIDENCE OF POTENTIAL DECK WEDING FAILURE.	SH-3003 TRANSVERSE ELEVATION BEALE STREET BRIDGE SH-3104 BRIDGE DECKING SECTIONS GRID LINES & BENTS	53
B.1. SEE SHEET SH-1100 FOR THE CONSTRUCTION PROCEDURE FOR THE INSTALLATION OF THE TEMPORARY BRIDGES.	GRADING REQUIREMENTS OF SECTION 39-2.02, "AGGREGATE", OF THE STANDARD SPECIFICATIONS.	IF THERE IS EVIDENCE OF POTENTIAL DECK WELDING FAILURE. B. SITE OBSERVATION	SH-3105 BRIDGE DECKING SECTIONS ABUTMENTS ON THE NORTH SIDE	\$13/
B.2. GENERAL CONTRACTOR SHALL RESOLVE ANY CONFLICT BETWEEN BRIDGE COLUMN/PIER/BRACING, STRUTS AND STRUCTURAL COLUMNS/PILE CAPS	D.5. AREA SUBJECT TO VEHICLE TRAVEL SHALL BE SURFACES PROVIDING A MINIMUM STATIC COEFFICIENT OF FRICTION EQUAL TO 0.65.	B.1. REPRESENTATIVE OF PB&A SHALL VISIT THE SITE AT LEAST ONCE A WEEK DURING TEMPORARY BRIDGE INSTALLATION AND EXCAVATION	SH-3106 BRIDGE DECKING SECTIONS ABUTMENTS ON THE SOUTH SIDE SH-4100 LONGITUDINAL SECTIONS FIRST & FREMONT STREET BRIDGES	PRINCIPAL ENGINEER
COLUMNY PIECY BRACING, SINUS AND SINUCUTURAL COLUMNS/ PILE CAPS PRIOR TO START OF CONSTRUCTION. C. PROCEDURE FOR INSTALLATION OF BRIDGE COLUMN PIERS	D.6.AREA SUBJECT TO PEDESTRIAN TRAVEL SHALL BE SURFACES PROVIDING A MINIMUM STATIC COEFFICIENT OF FRICTION EQUAL TO	OPERATIONS TO PROVIDE STRUCTURAL OBSERVATION IN ACCORDANCE WITH THE SAN FRANCISCO BUILDING CODE. PB&A SHALL ISSUE A WRITTEN REPORT FOR EACH SITE VISIT.	SH-4101 LONGITUDINAL SECTION BEALE STREET BRIDGE	PROJECT MANAGER P.
C. TROOLDOKE FOR INSTALLATION OF BRIDGE CULUMIN PIERS	0.65 WHERE SLOPES ARE LESS THAN OR EQUAL TO 5%. AND 0.80		SH-5100 DETAILS	Q. LIU P. BARAR
C.1. DRILL 48"Ø HOLE FROM GROUND LEVEL TO THE DESIGNED TIP FLEVATION	WHERE SLOPES EXCEED 5%.	C. MONITORING		
C.1. DRILL 48"\$ HOLE FROM GROUND LEVEL TO THE DESIGNED TIP ELEVATION. A 60"\$ X 20"-0" "STARTER" CASING IS USED IN THE UPPER 20 FT OF LOOSE MATERIAL HOLE IS SUPPORTED BY DRILLING SLURRY (WATER AND	WHERE SLOPES EXCEED 5%. E. RAILING	C. MONITORING C.1.PLACE A SURVEY MONITORING POINT ON THE BRIDGE DECK AT EACH	SH-5101 DETAILS SH-5102 DETAILS	ML/PS/CP 05/02/201 SOLC SET FADURY NO.
A 60"\$ X 20'-0" "STARTER" CASING IS USED IN THE UPPER 20 FT OF				

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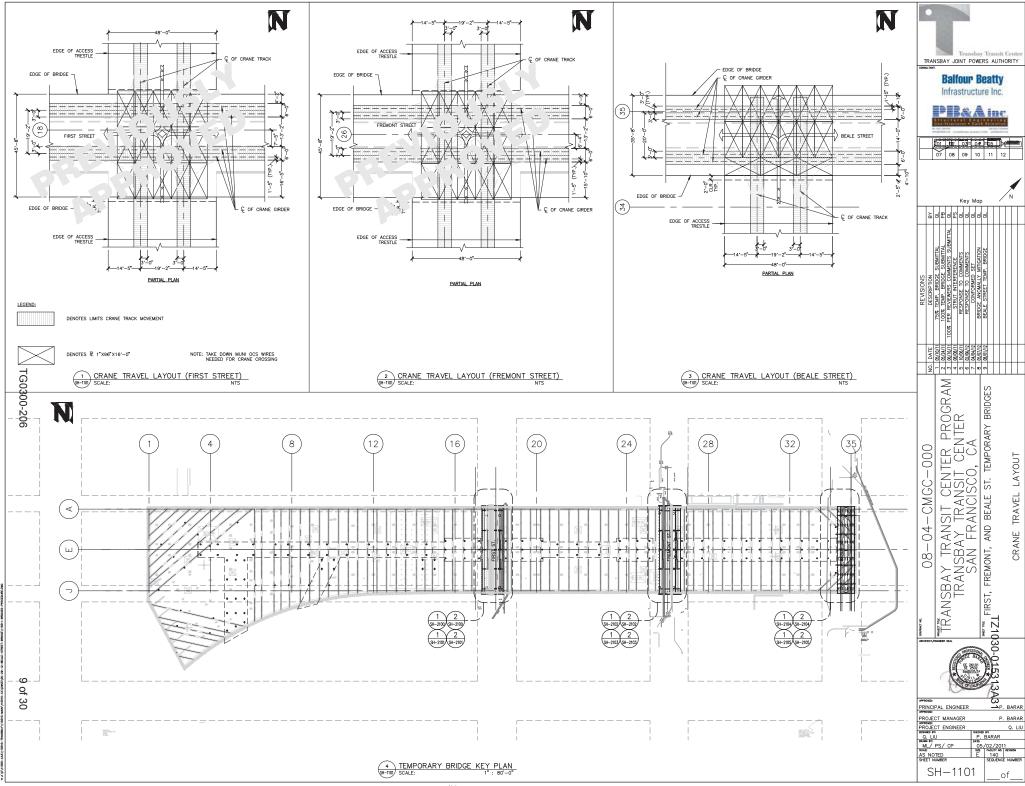
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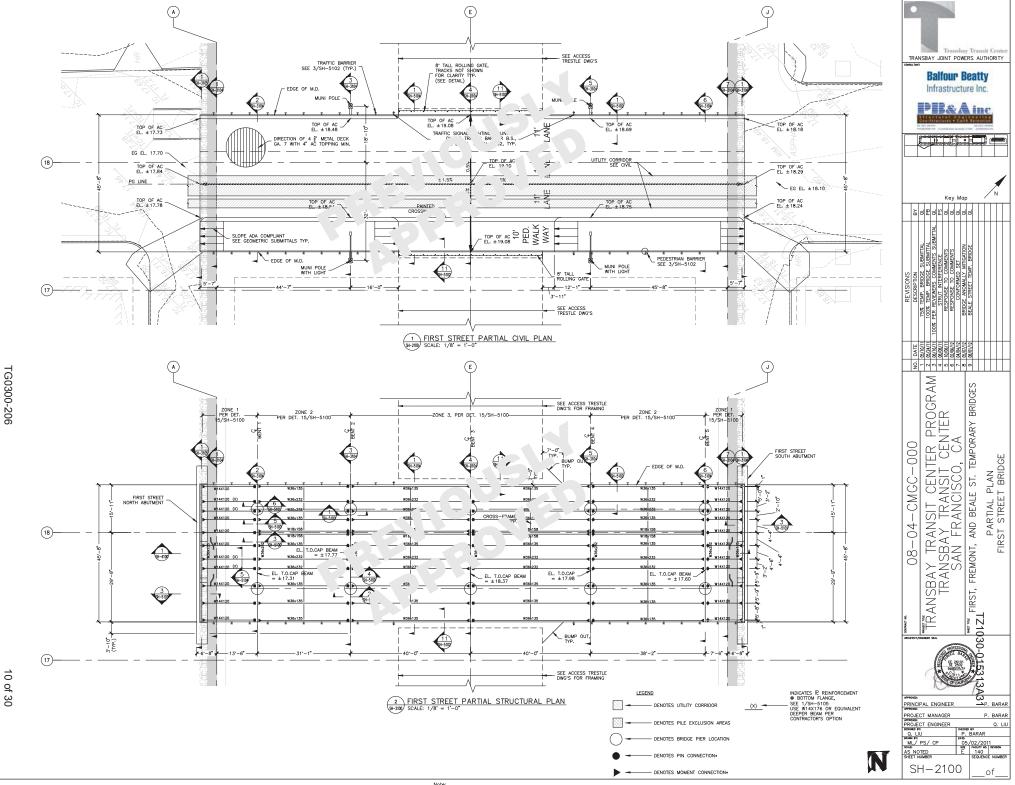




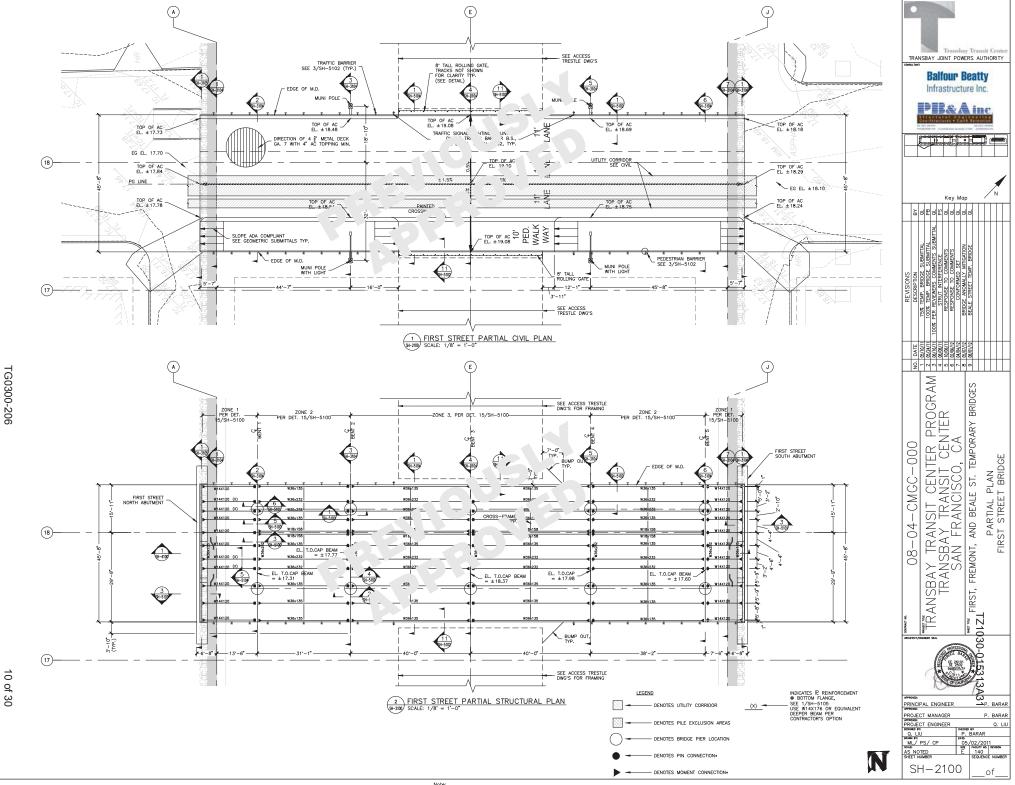
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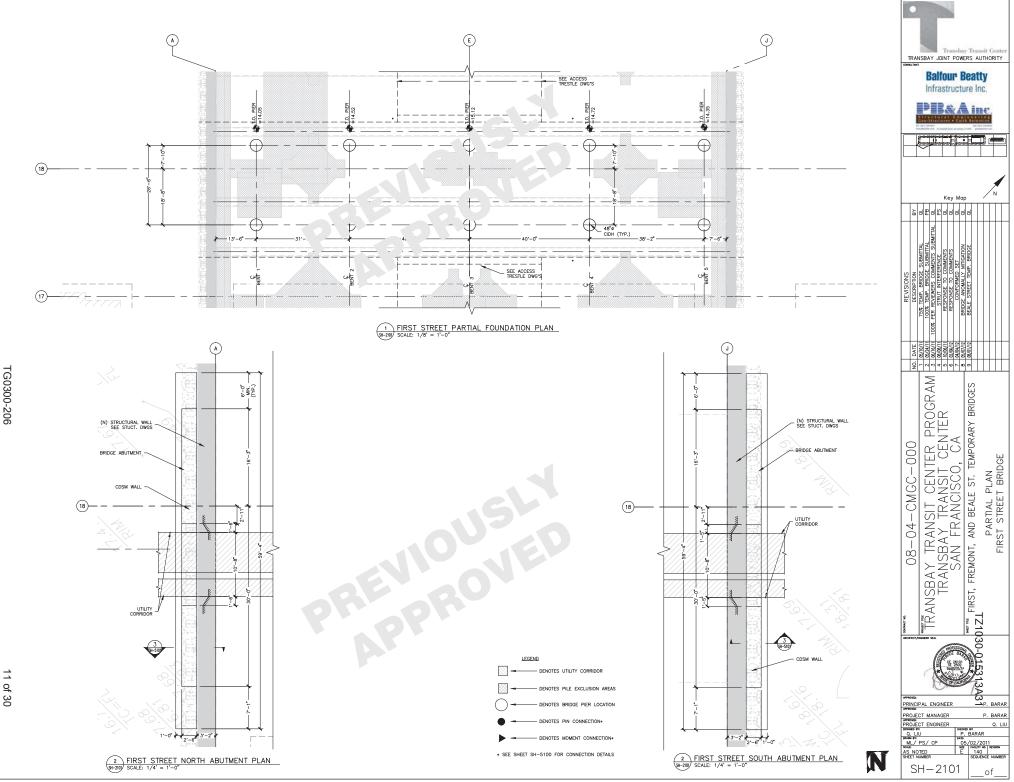
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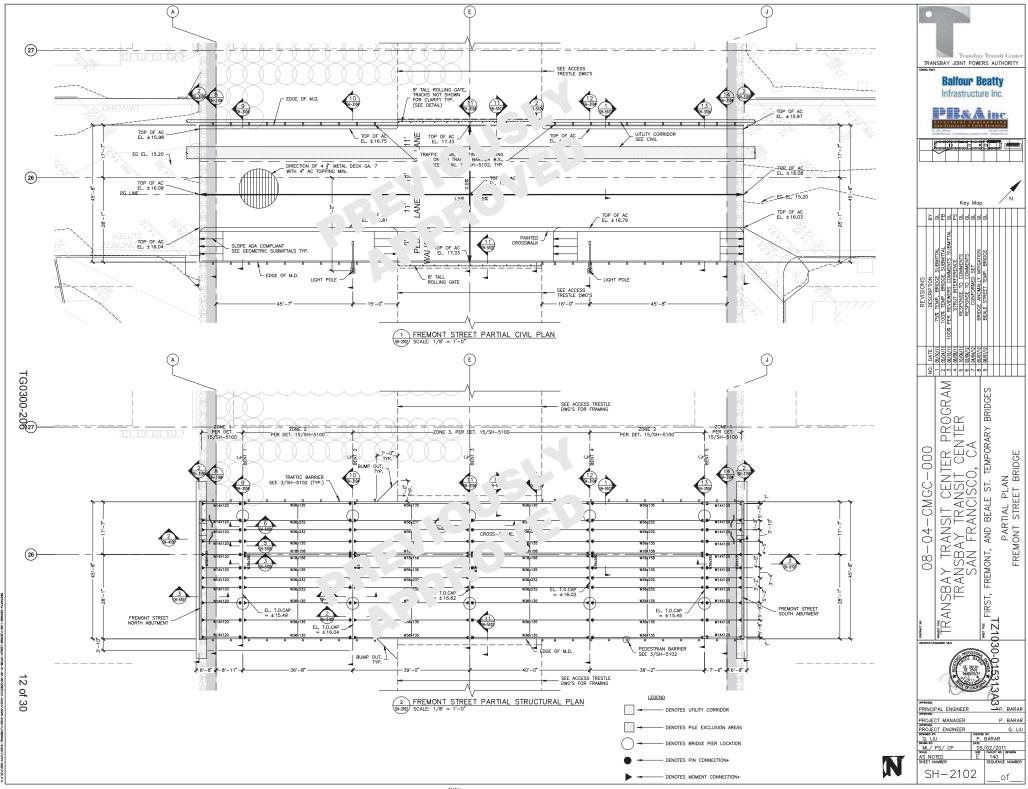
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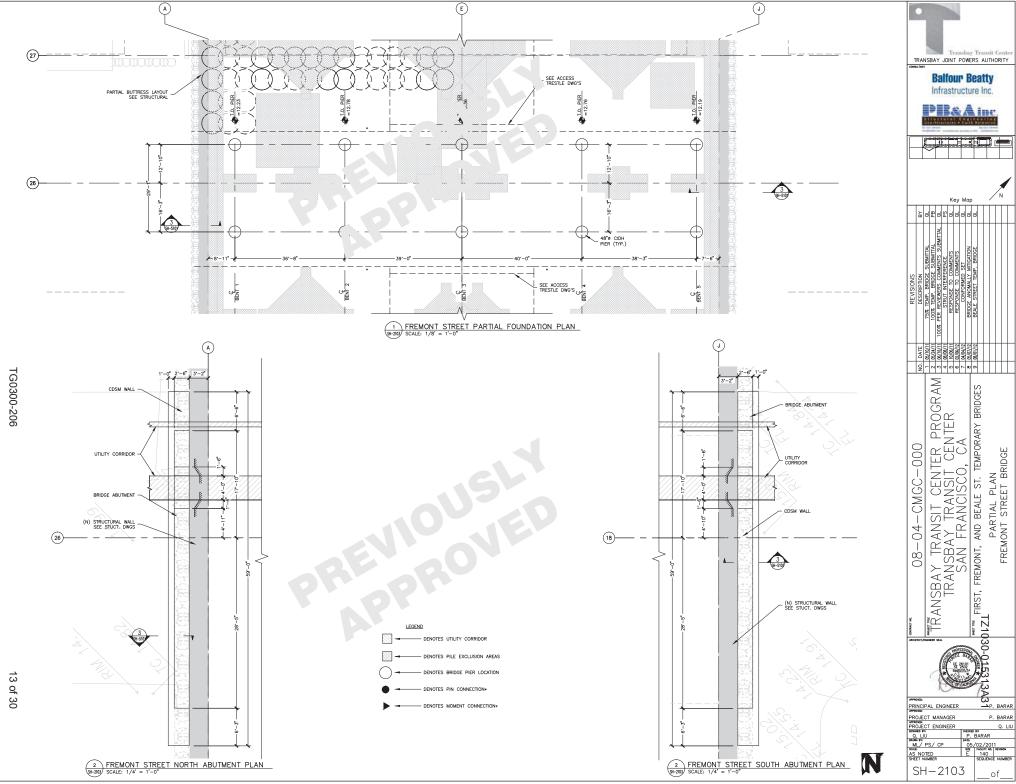
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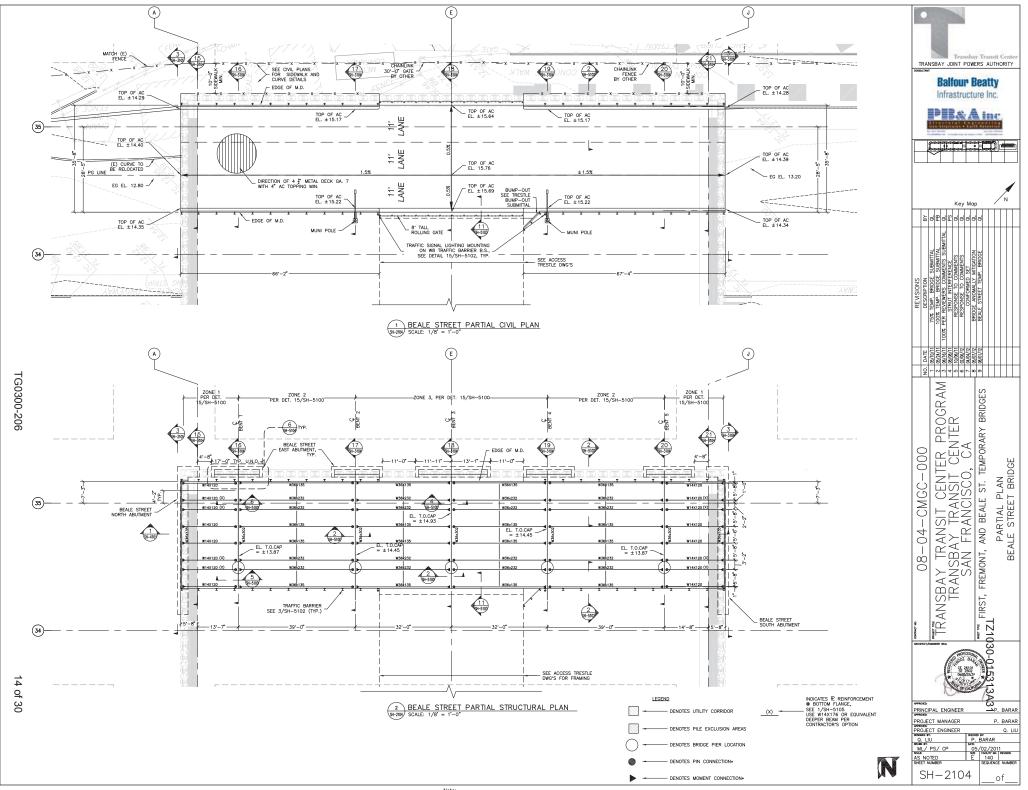
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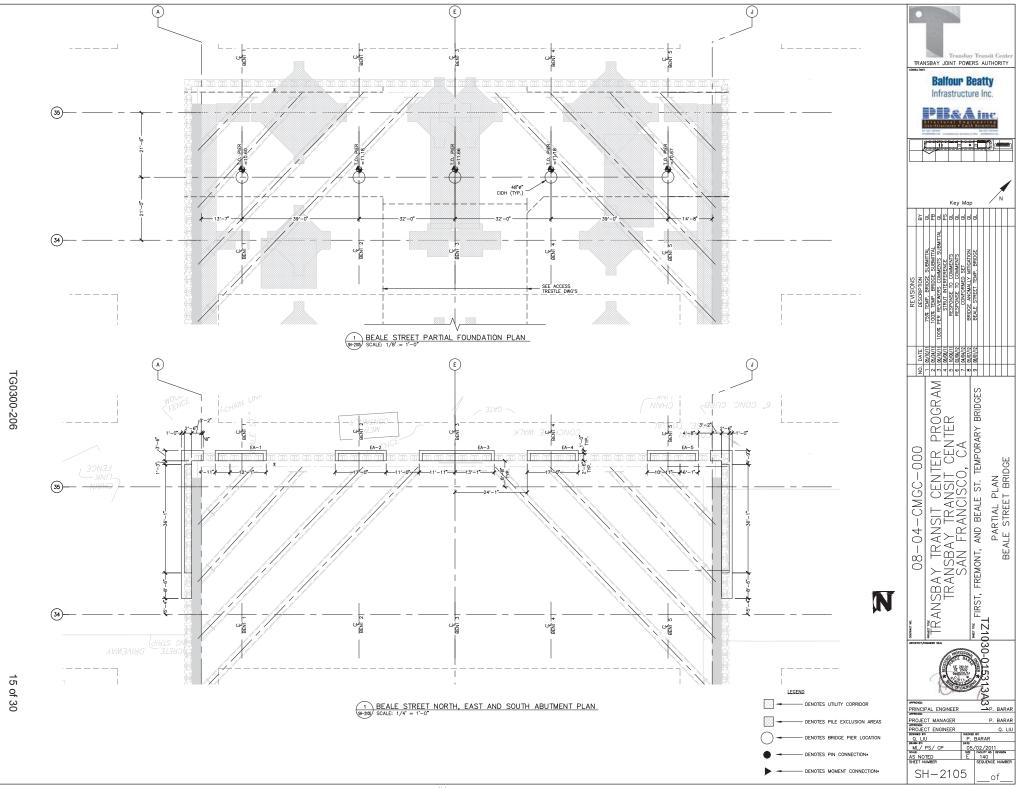
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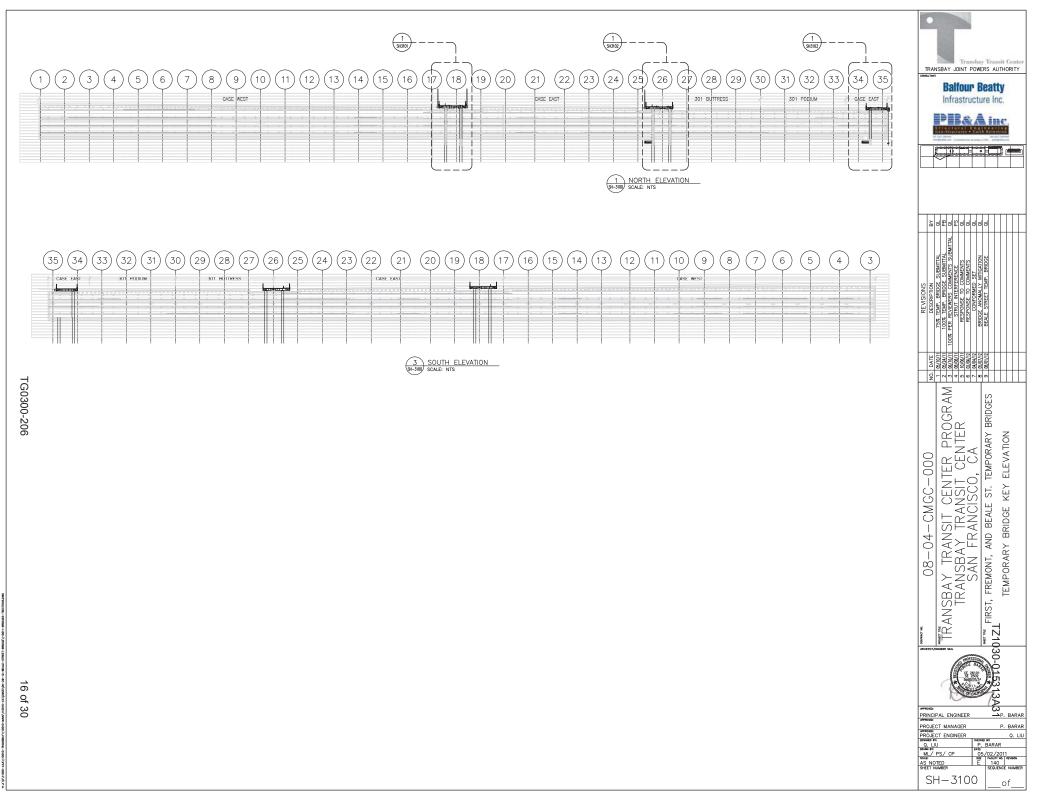


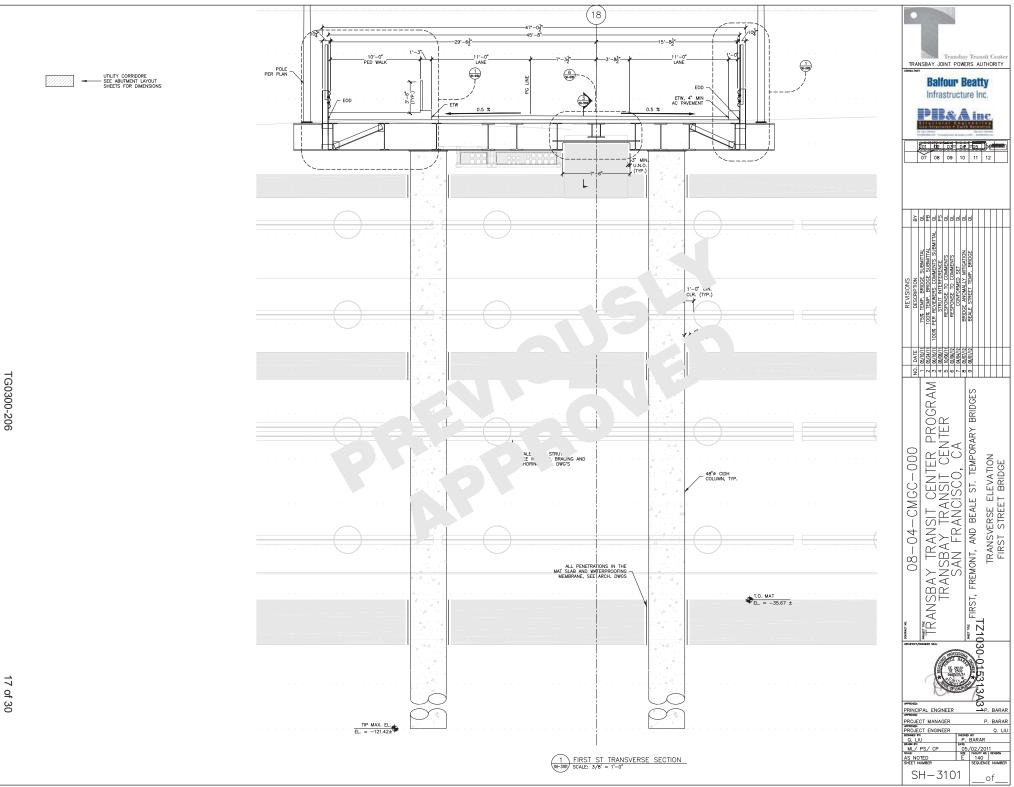
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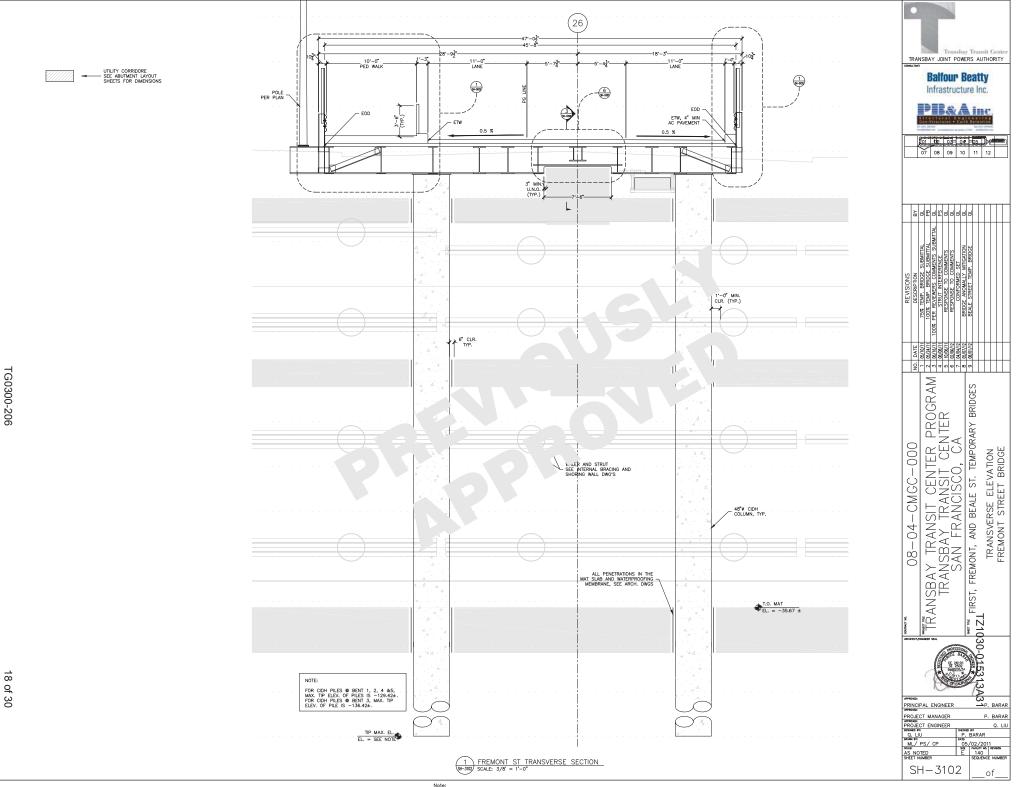
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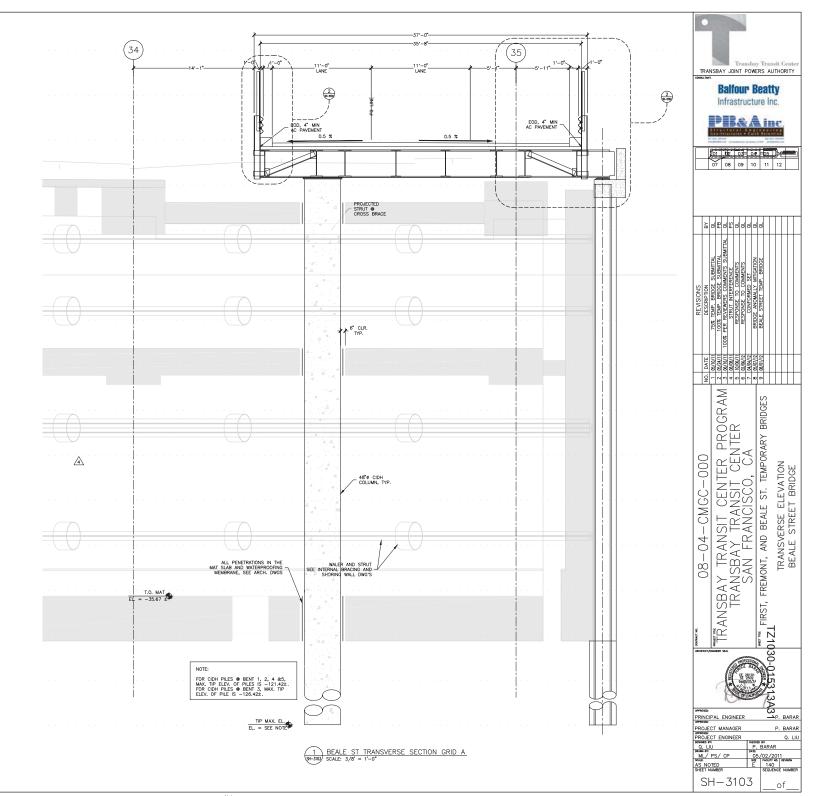
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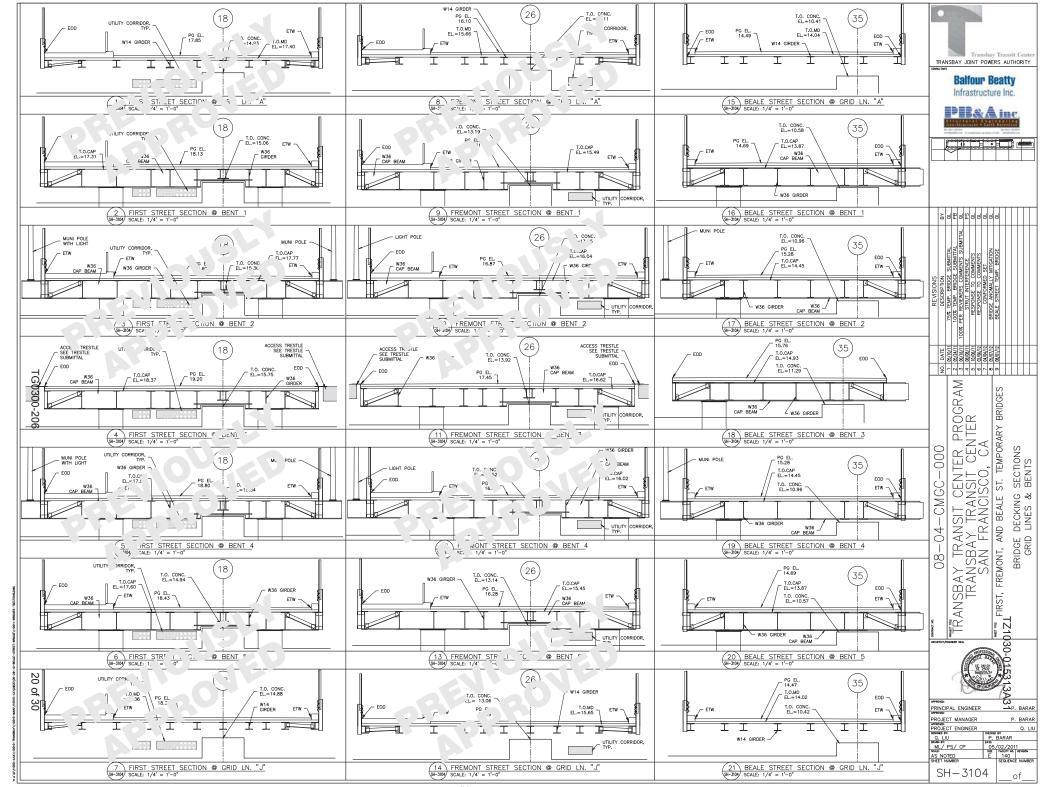


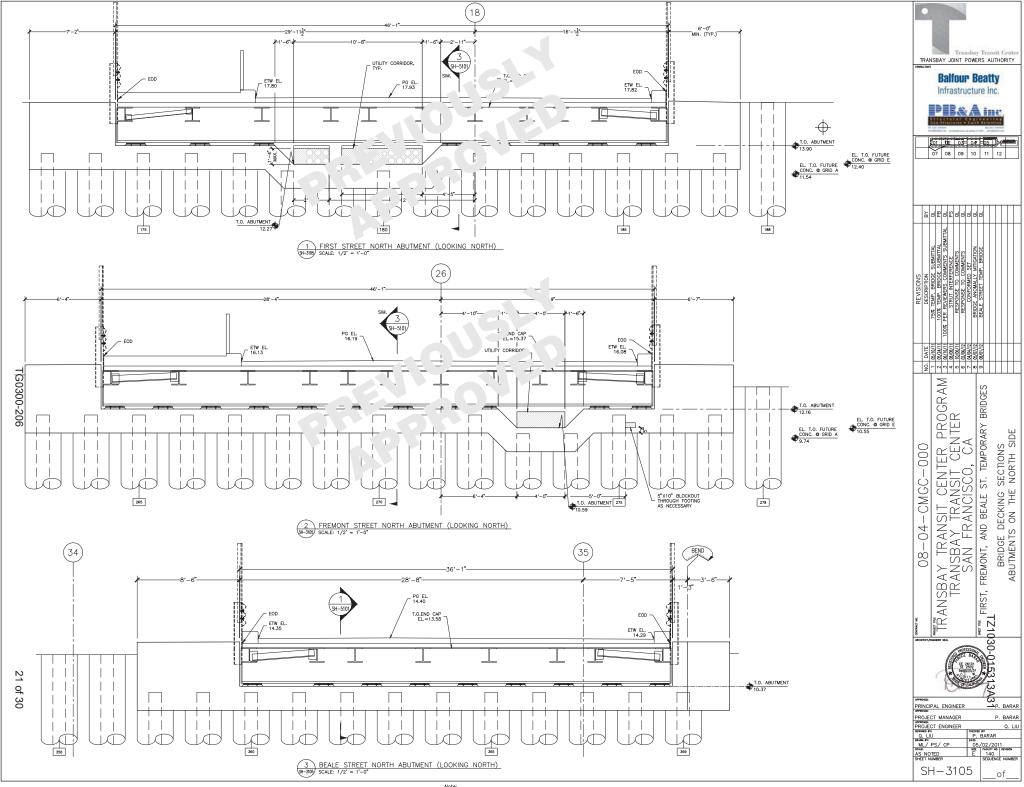
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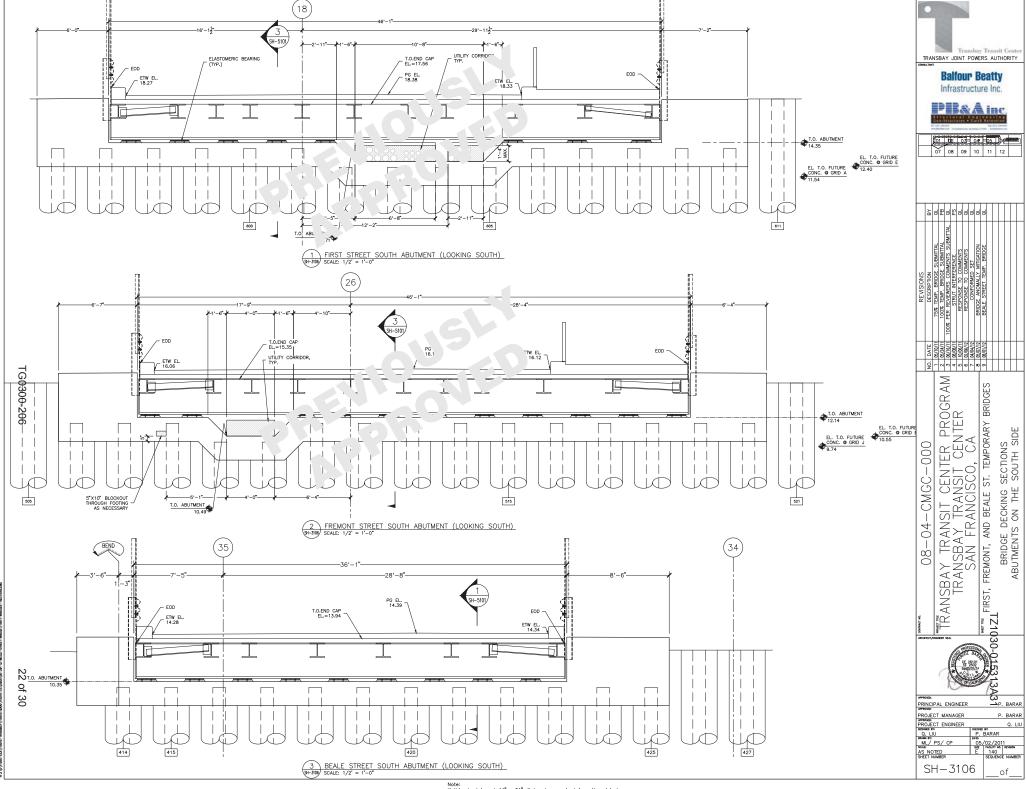
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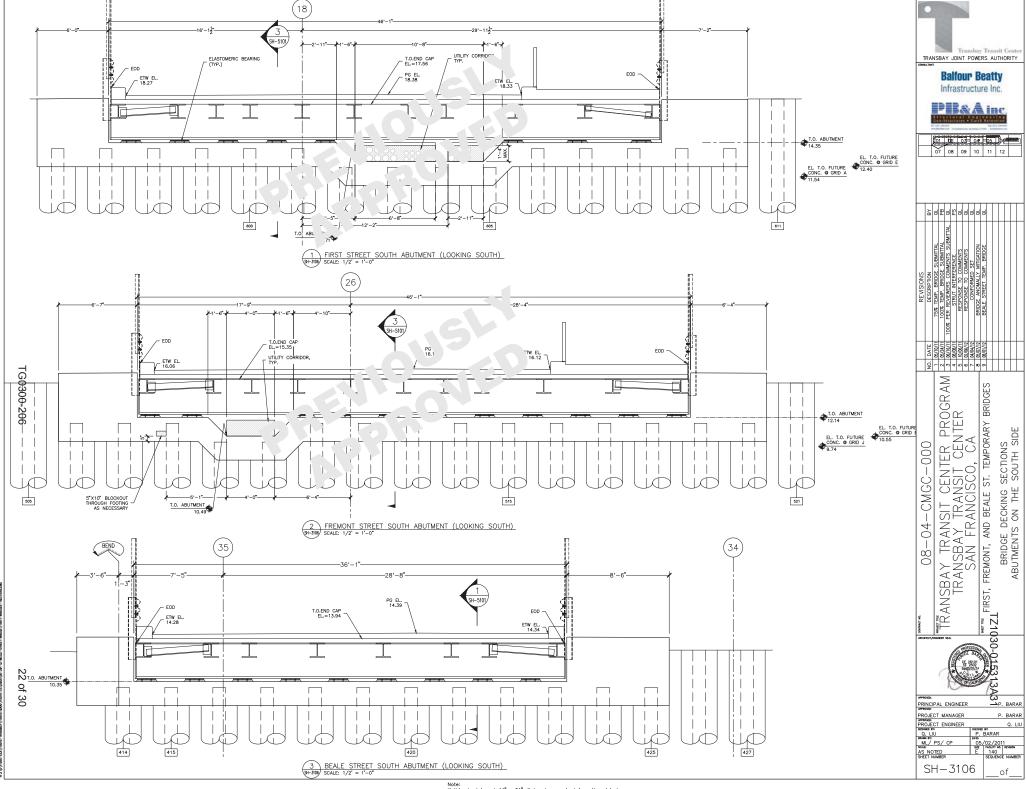


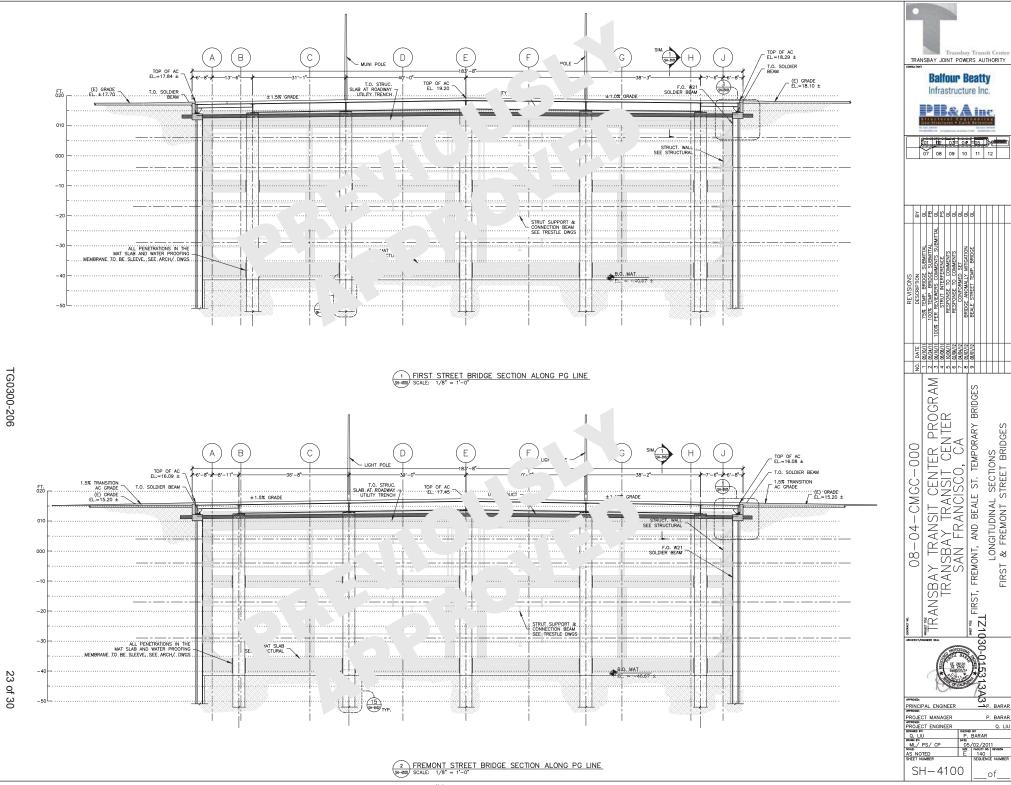




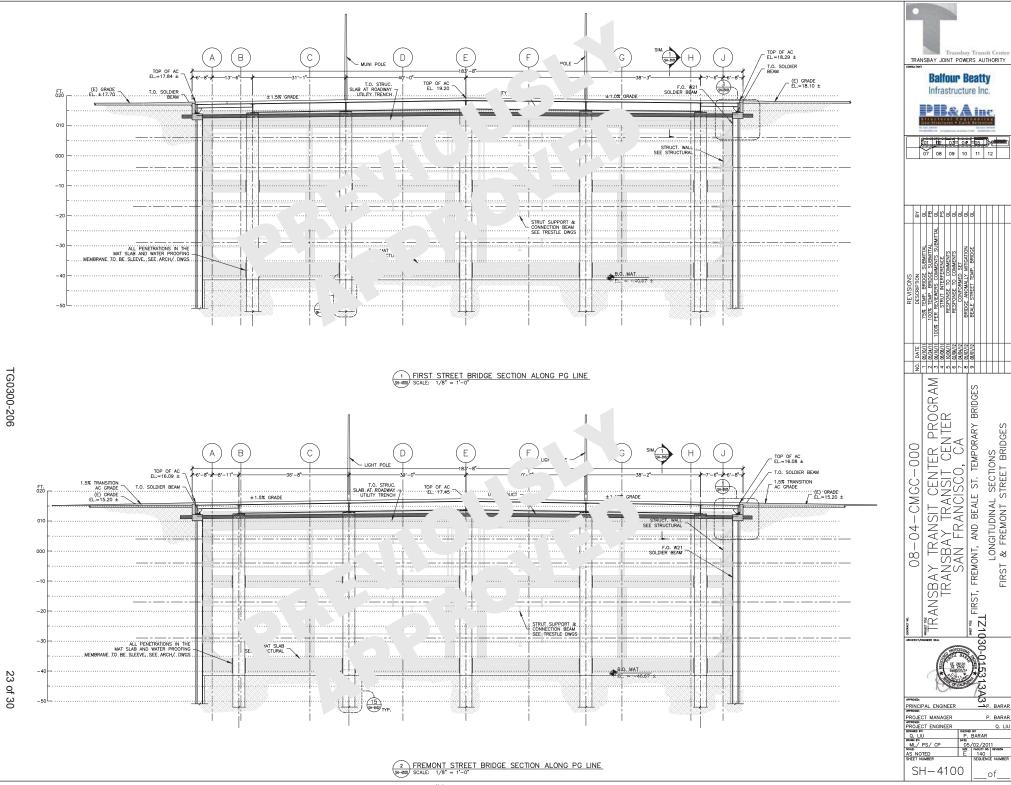




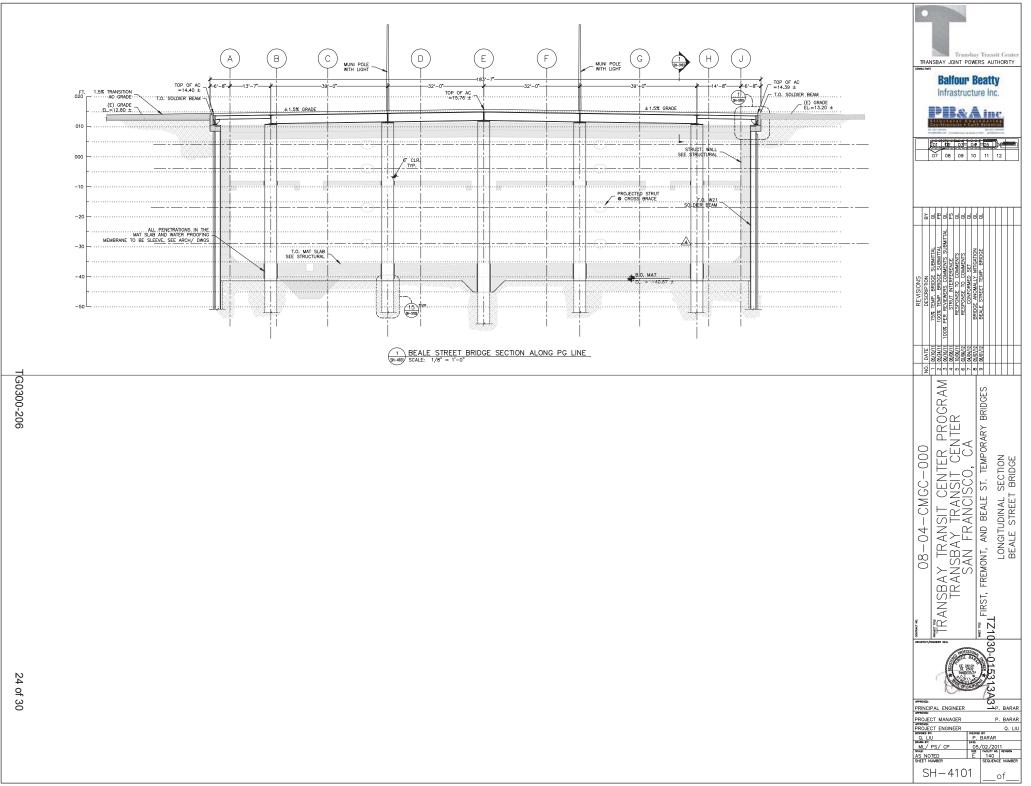


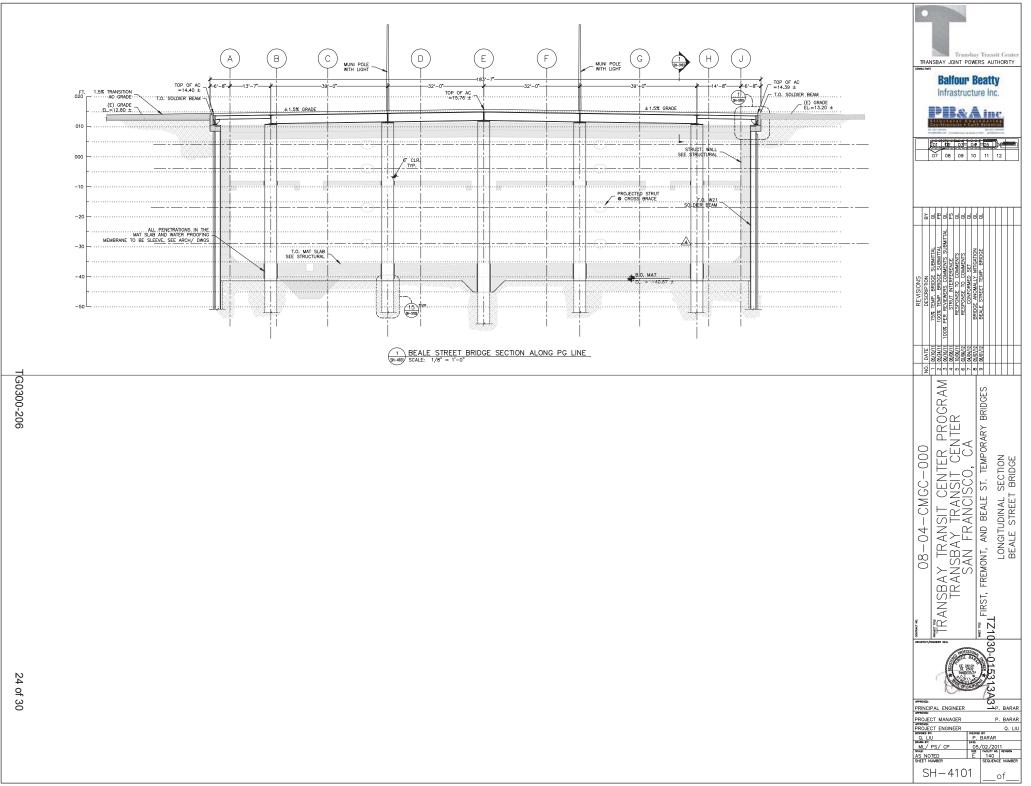


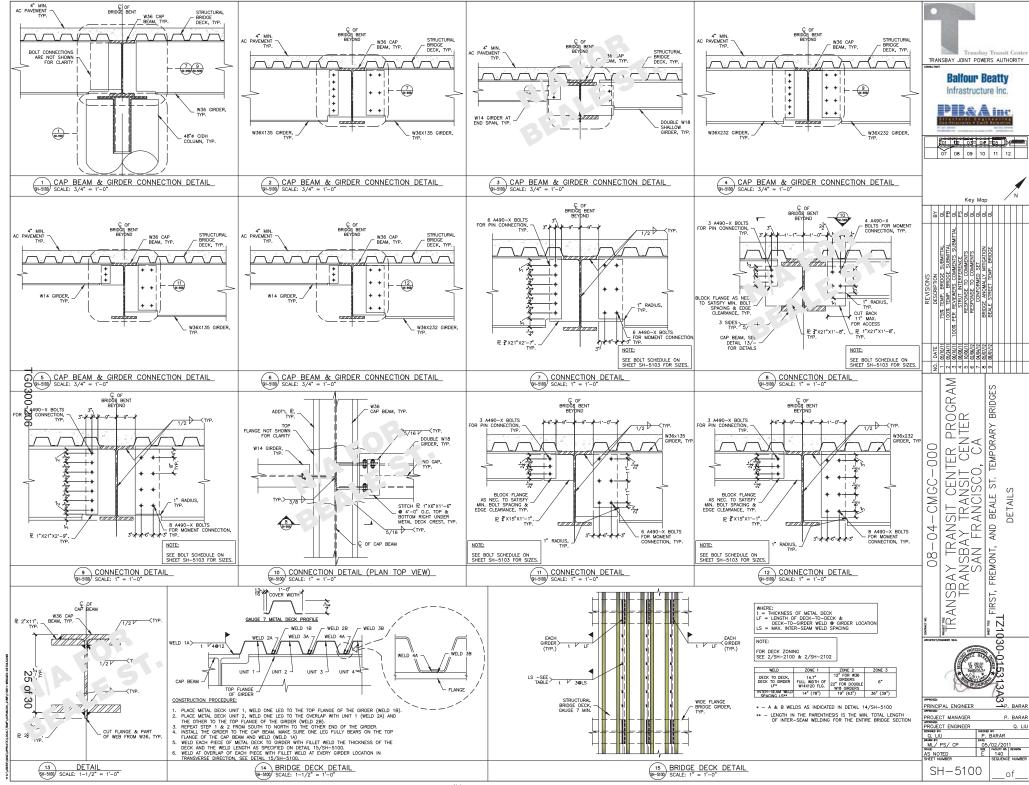
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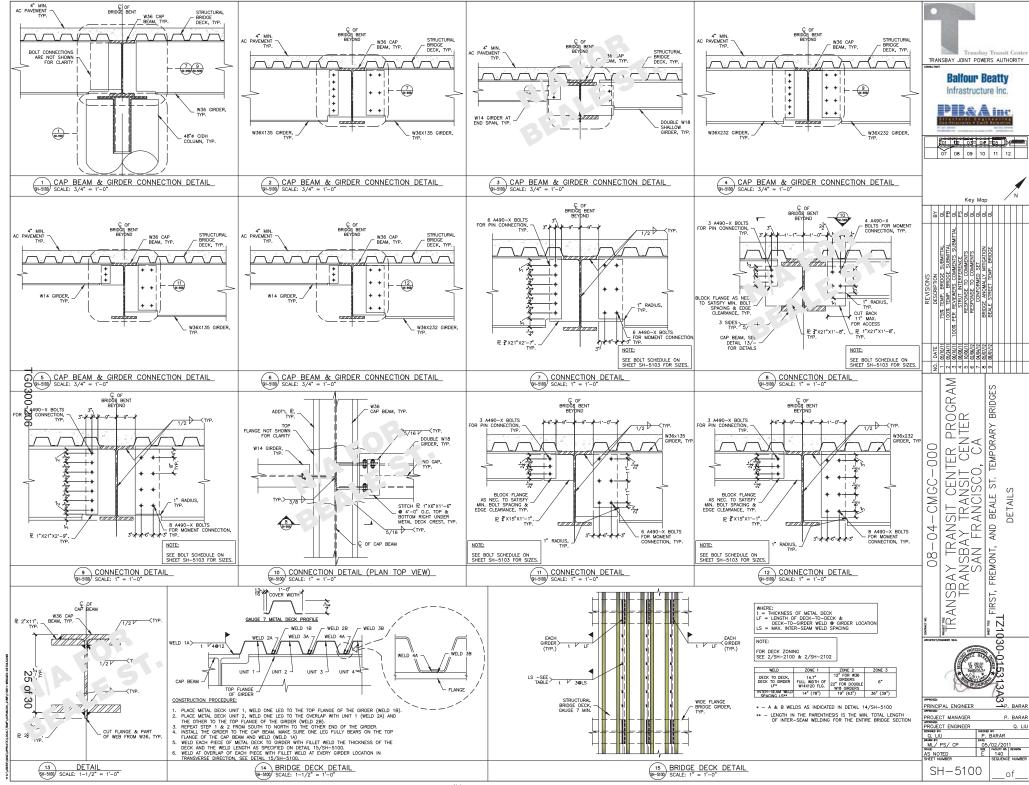
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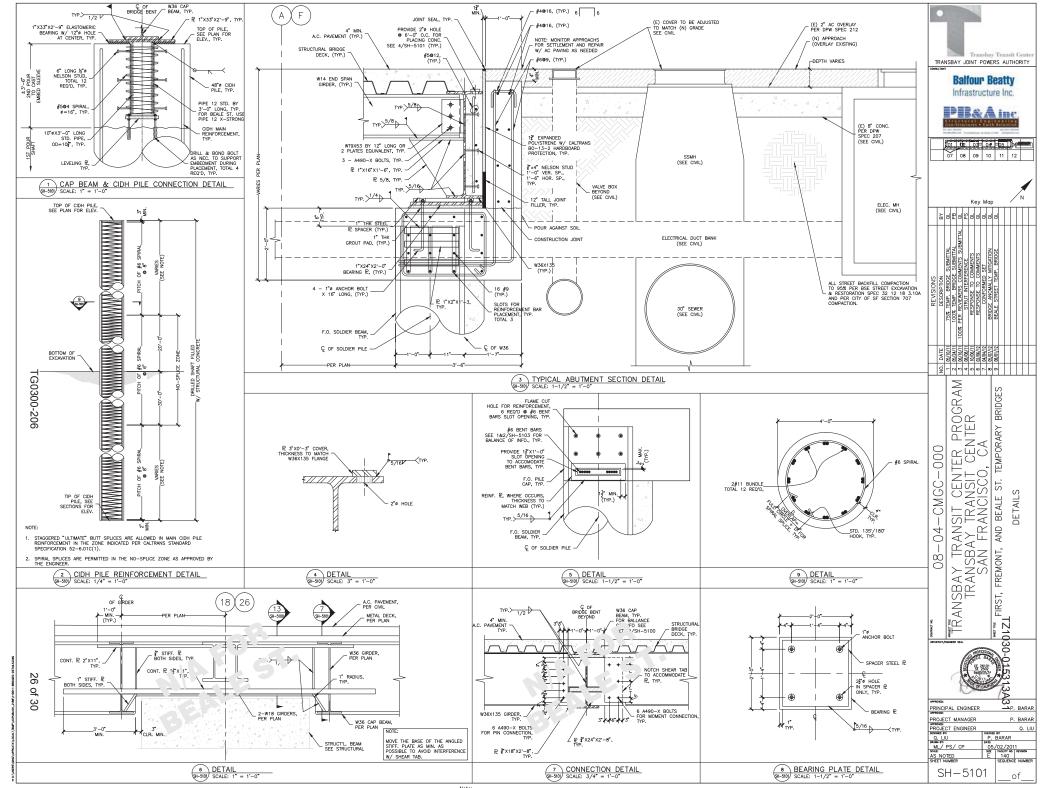


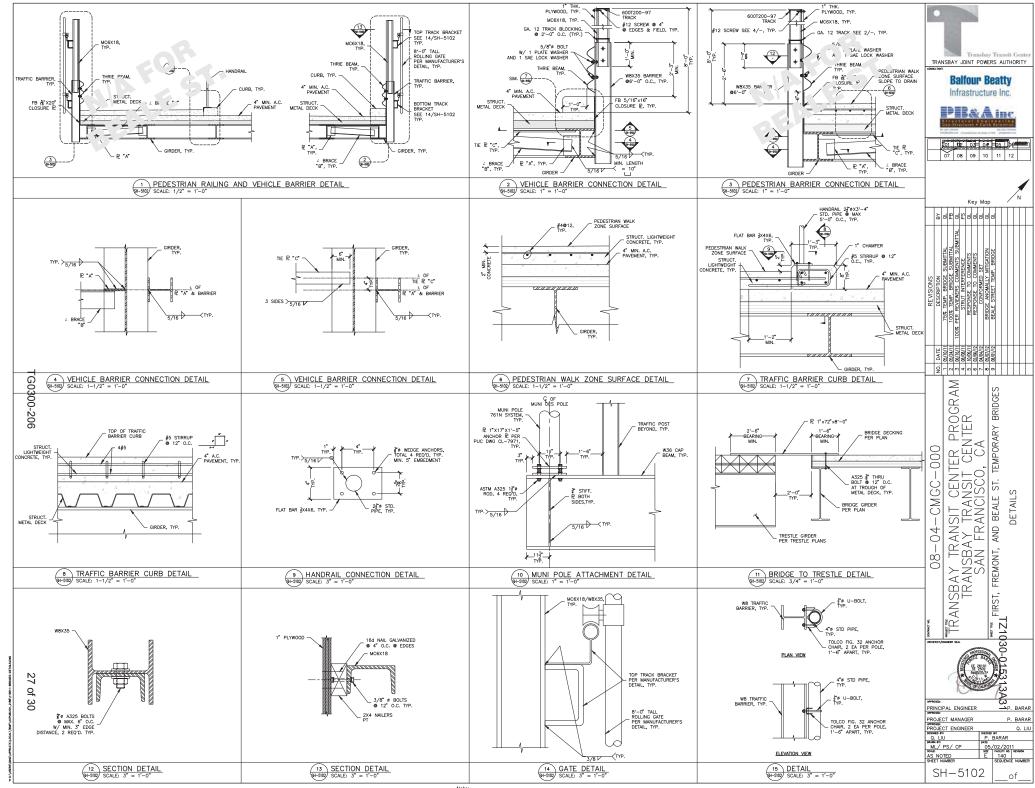


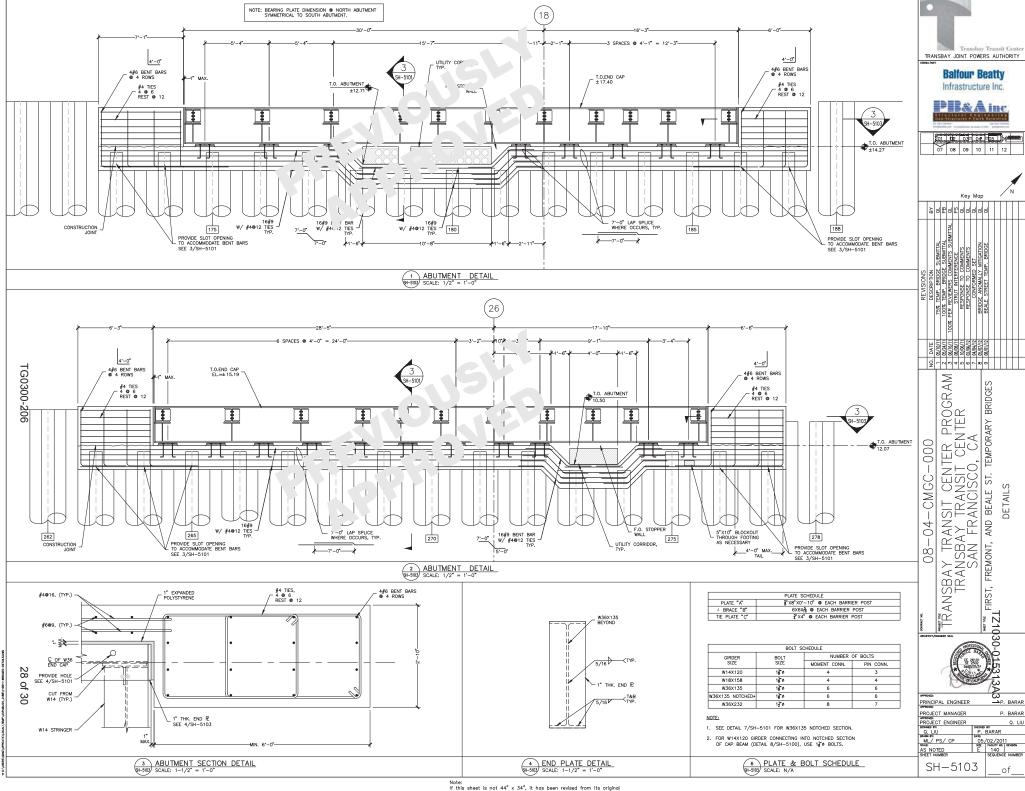
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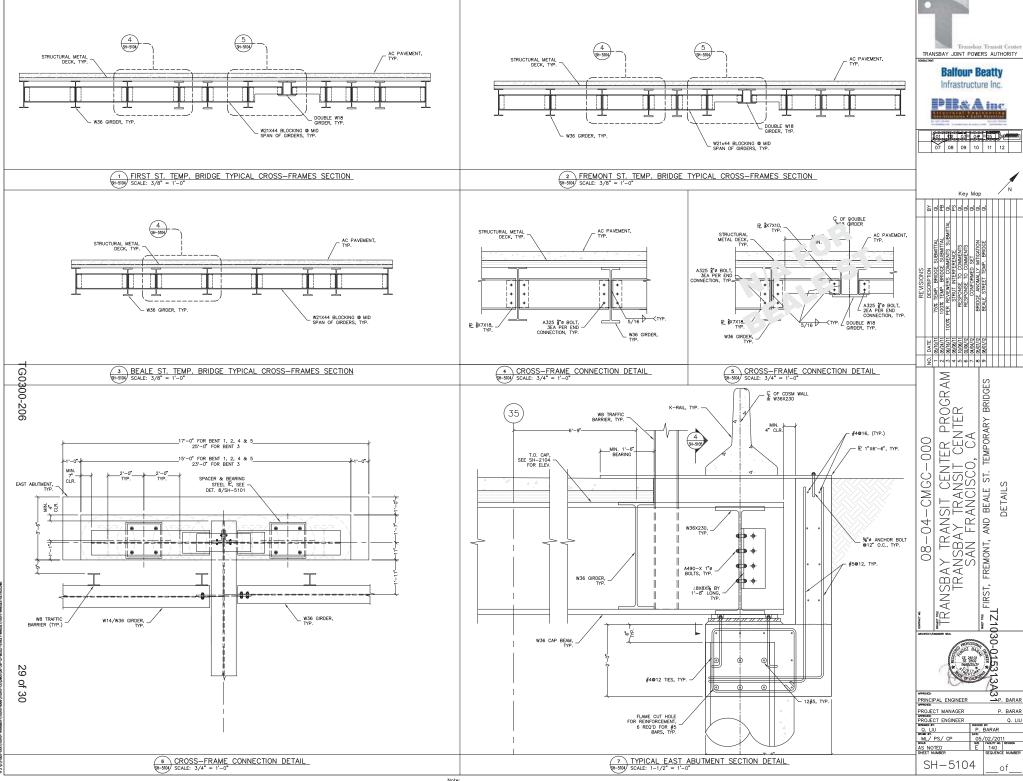
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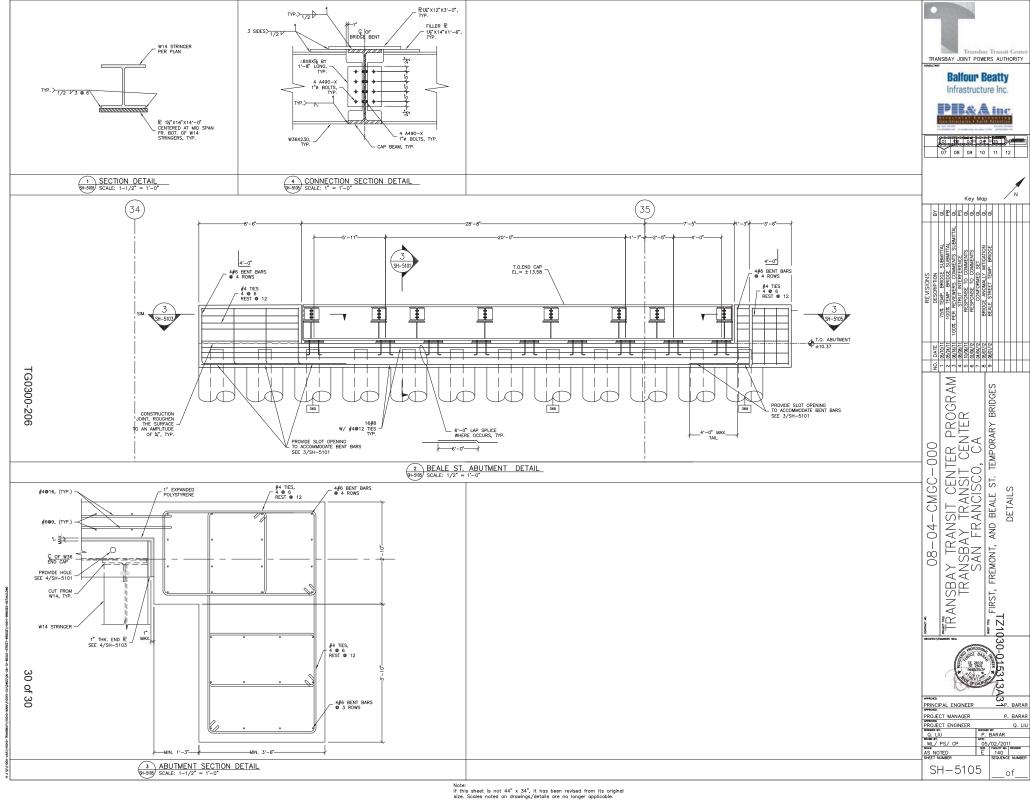




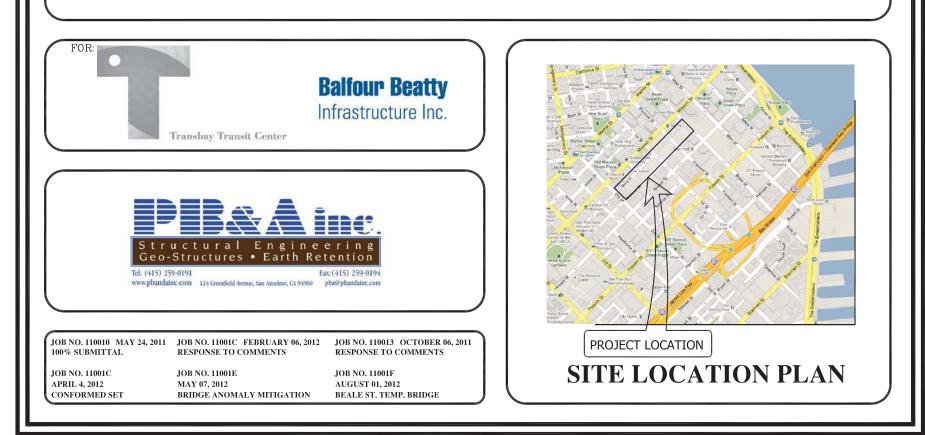
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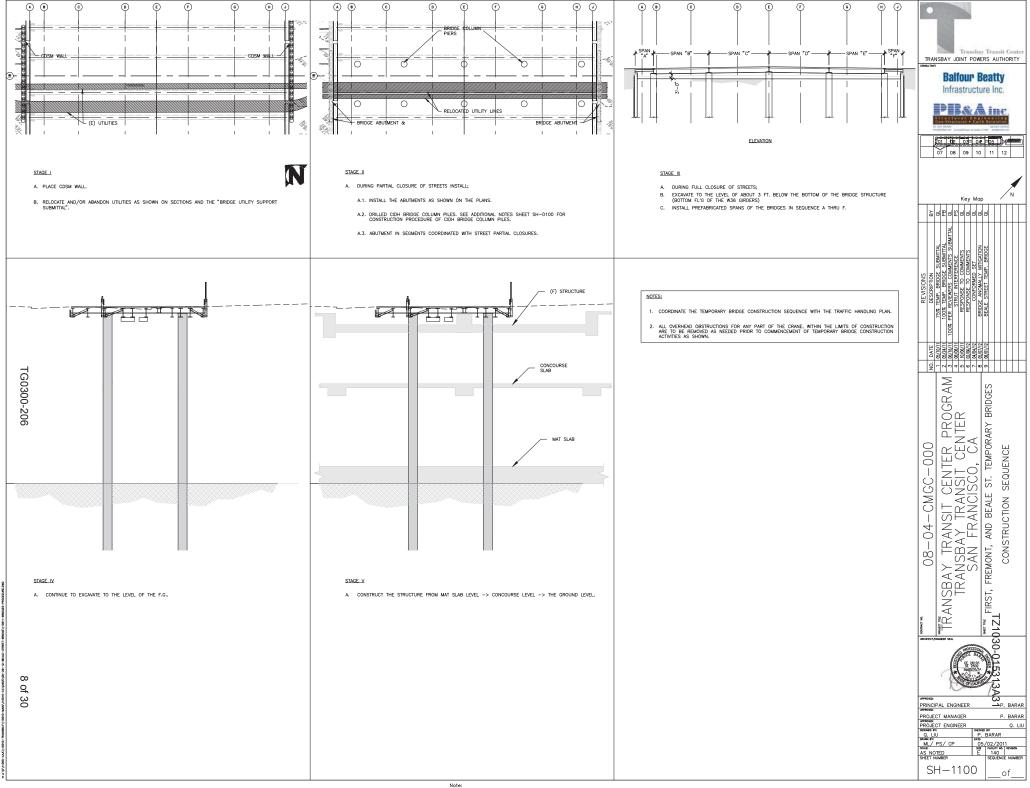
FIRST, FREMONT AND BEALE STREET TEMPORARY BRIDGE STRUCTURAL DESIGN TRANSBAY TRANSIT CENTER PROGRAM TRANSBAY TRANSIT CENTER, SAN FRANCISCO, CA

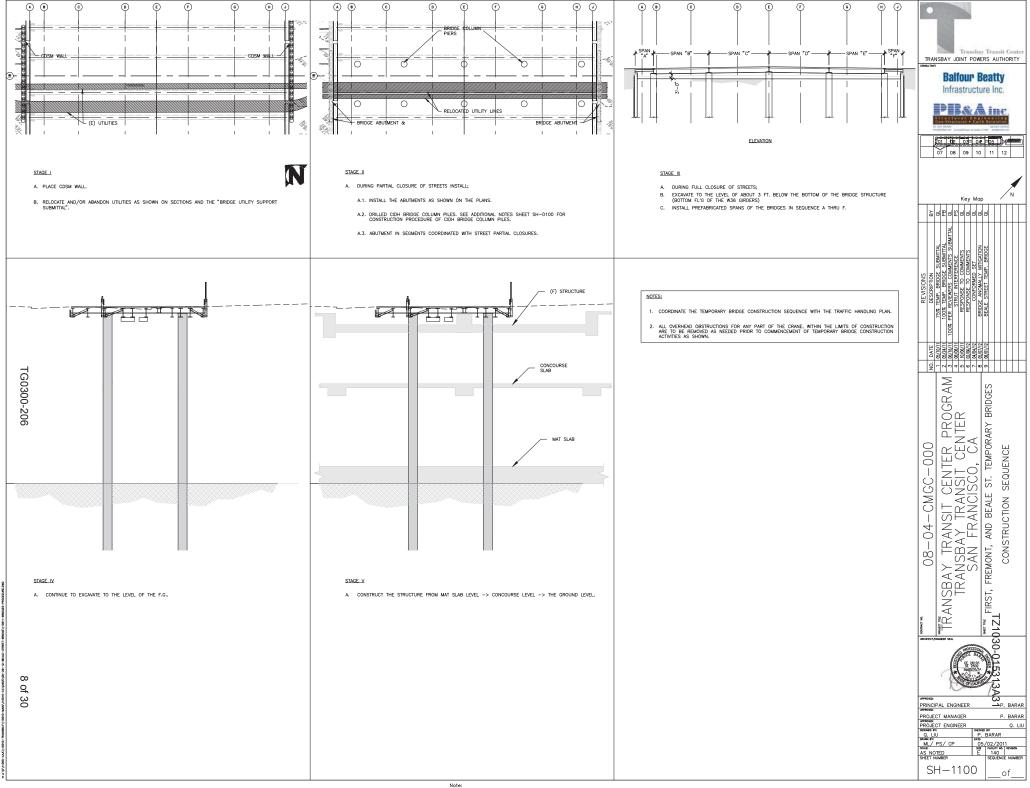


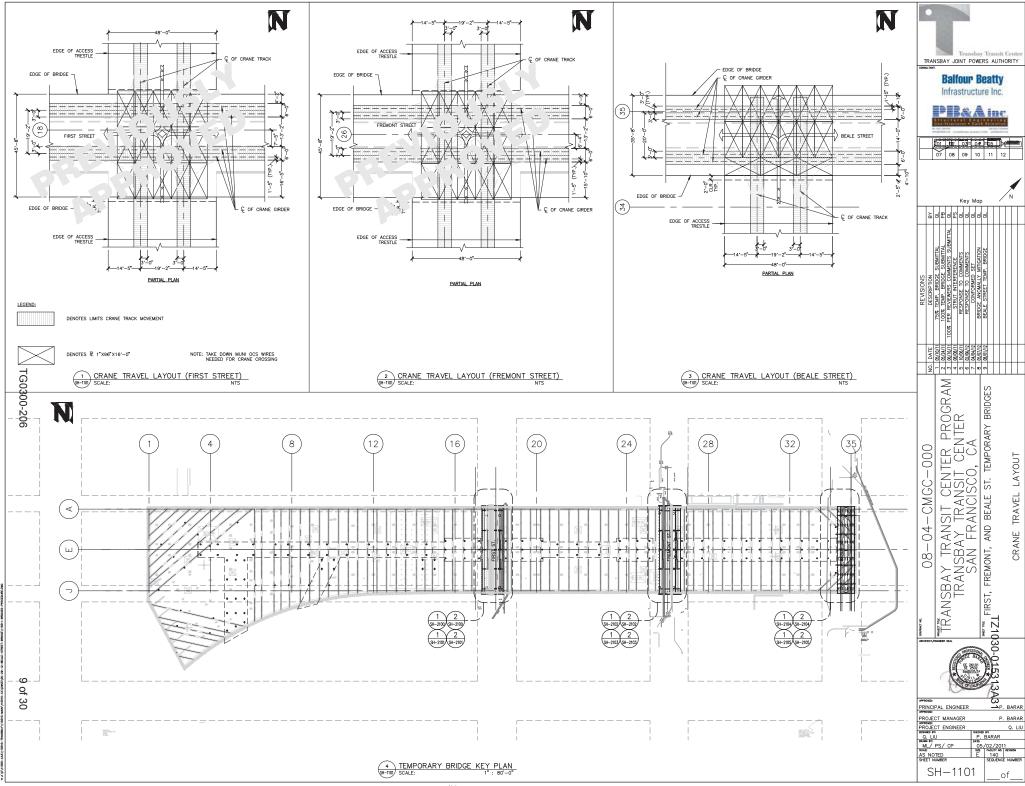
GENERAL NOTES	GENERAL NOTES	GENERAL NOTES	GENERAL NOTES	•
ART 1 - GENERAL	C.3. PLACE REINFORCEMENT CASE IN THE HOLE. USE RIGGING TO SECURE REINFORCEMENT CAGE TO CASING DURING CONCRETE PLACEMENT.	E.3.METAL STUD & TRACK: ASTM A653, MIN. G40 COATING		
 SCOPE OF WORK A. IN ORDER TO ALLOW CONTINUATION OF TRAFFIC ACROSS THE CONSTRUCTION SITE EXCAVATION AT FIRST, FREMONT, AND BEALE STREETS FOR A TOTAL OF THREE (3) BRIDGES. BRIDGES AT 	C.4. PLACE CONCRETE BY TREMIE METHOD FROM TIP ELEVATION TO THE TRANSITION CONSTRUCTION JOINT AS INDICATED ON THE PLANS. AS THE CONCRETE IS PLACED SLURRY IS TO BE PUMPED BACK INTO SLURRY TAMKS	F. PLAIN ELASTOMERIC BEARING F.1.THE MATERIAL SPECIFICATIONS SHALL MEET ALL REQUIREMENTS OF ANSHTO M251.		Transbay Transit Cen TRANSBAY JOINT POWERS AUTHORIT
EACH CROSSING SHALL PROVIDE A CONNECTION ACROSS THE CONSTRUCTION EXCAVATION, FROVIDING LANES ON BRIDGES TO PROVIDE A PATH FOR VEHICLES AND PEDESTRIANS ACROSS THE CONSTRUCTION SITE.	C.5. THE RIGGING IS TO REMAIN IN PLACE UNTIL THE CONCRETE SETS.	F.2. PLAIN ELASTOMERIC BEARINGS SHALL CONFORM TO SECTION 51-1.12H(1) OF CALTRANS STANDARD SPECIFICATIONS.		Balfour Beatty
B. RELATED WORK NOT INCLUDED IN THIS PACKAGES:	C.7. INSTALL THE CONNECTION ELEMENTS AT THE TOP OF BRIDGE COLUMN PILE			Infrastructure Inc.
B.1. CONCRETE BASEMENT WALL,	AS INDICATED ON THE PLANS. C.8. PREPARE THE CONSTRUCTION JOINT.	SURFACE OF THE PTFE AND THEIR DEPTH SHALL BE NOT LESS THAN 0.08" AND NOT MORE THAN HALF THE THICKNESS OF THE PTFE. THE		
 B.2. SHORING OR BRACING FOR EXISTING BUILDINGS, B.3. TRESTLES, 	C.9. POUR CONCRETE TO THE DESIGNED TOP OF PILE ELEVATION.	RESERVOIRS SHALL BE UNIFORMLY DISTRIBUTED OVER THE SURFACE AREA AND SHALL COVER MORE THAN 20% BUT LESS THAN 30% OF IT. LUBRICANT SHALL BE SILICONE GREASE WHICH SATISFIES MILITARY SPECIFICATION MIL-S-B660.		
B.4. STRUT REMOVAL AND REBRACING		G. POLYETHYLENE VAPOR BARRIER: ASTM E1745 CLASSES A, B, & C		
B.5. PLANNED CONSTRUCTION, OR EXCAVATION OTHER THAN NOTED ABOVE.	A. CONCRETE AT DRILLED SHAFTS AND ABUTMENTS A.1. PORTLAND CEMENT ASTM C-150 TYPE "I" OR TYPE "IA"	H. PLYWOOD: APA RATED CD-X, STRUC I		
B.6. BRIDGE TRAFFIC: SIGNAL & SIGNAGE	A.1.PORILAND CEMENT ASTM C-ISO TYPE T OR TYPE TA A.2.NORMAL WEIGHT CONCRETE = 3.500 PSI AT 28 DAYS, PROPORTIONED	PART 3 - Q/A PROGRAM		
B.7. BRIDGE LIGHTING B.8. BRIDGE MUNI OCS	WITH A MAXIMUM WATER CEMENT RATIO, W/C=0.50. ALL CONCRETE SHALL BE BATCHED, DELIVERED, AND PLACED IN ACCORDANCE WITH	3.1. SPECIAL INSPECTION, INSPECTION PROCEDURES AND SITE OBSERVATION		
B.9. UTILITY SUPPORT DETAILS	ACI 301 AND ASTM C94.	A. SPECIAL INSPECTIONS		
B.10. TRAFFIC CONTROL PLANS	A.3. SUBMIT MIX DESIGN PREPARED BY A PROFESSIONAL ENGINEER REGISTERED IN THE STATE OF CALIFORNIA FOR ENGINEER'S APPROVAL.	A.1.THE SPECIAL INSPECTORS MUST BE CERTIFIED BY TJPA TO PERFORM THE TYPE OF INSPECTION SPECIFIED.		Key Map
C. RELATED DOCUMENT:	B. STRUCTURAL LIGHTWEIGHT CONCRETE AT PEDESTRIAN WALK	A.2.SPECIAL INSPECTIONS SHALL BE PROVIDED BY THE TJPA FOR		≿ 역 또 역 또 역 역 역 역 역 역
C.1. "FINAL GEOTECHNICAL DATA REPORT" PREPARED BY ARUP, DATED FEBRUARY 2010	B.1.PORTLAND CEMENT ASTM C-150 TYPE "I" OR TYPE "IA	PERMANENT AND TEMPORARY WORK ACCORDING TO THE CALIFORNIA BUILDING CODE (CBC) CHAPTER 17 INCLUDING THE SAN FRANCISCO		7
C.2. CONTRACT 08-04-CMGC-000 DWG'S, REV. 0 DATED 12-10-10. C.3. STRUCTURAL COMBINED.DWG - DATED 9/01/10	B.2.AGGREGATE: EXPANDED SHALE, CLAY, OR SLATE (ESCS) LIGHTWEIGHT AGGREGATE PRODUCED BY THE ROTARY KILN METHOD SHALL MEET ASTM (230.	AMEDIDMENTS, AND THE PROJECT SPECIFICATIONS. THE SPECIAL INSPECTOR SHALL DBSERVE THE WORK FOR CONFORMANCE WITH THE CONSTRUCTION DOCUMENTS, THE SPECIAL INSPECTOR SHALL SEND REPORTS TO THE OWNER, ARCHITECT, REGNECER, BUILDING OFFICIAL		
C.4. XSURVEY.DWG - DATED 7/27/2010	B.3.GROUND GRANULATED BLAST-FURNACE SLAG SHALL MEET ASTM C989.	REPORTS TO THE OWNER, ARCHITECT, ENGINEER, BUILDING OFFICIAL AND THE CONTRACTOR IN A TIMELY MANNER. ALL DISCREPANCIES SHALL BE BROUGHT TO THE ATTENTION OF THE CONTRACTOR FOR		
C.5. XTRAF-SIGN.DWG - DATED 7/27/2010		CORRECTION, WHEN WORK IS DONE TO THE SATISFACTION OF THE INSPECTOR, THEN THE SPECIAL INSPECTOR SHALL SUBMIT A FINAL		AP. EST
C.6. RESPONSE TO RFI T-0073.1 DATED APR. 14, 2011: BSE - REQUEST FOR RESPONSE SPECTRA	B.4.MATERIALS SHALL BE PROPORTIONED TO PRODUCE CONCRETE WITH A MINIMUM COMPRESSIVE STRENGTH = 3,000 PSI IN 28 DAYS.	SIGNED REPORT STATING THAT, TO THE BEST OF THEIR KNOWLEDGE, THE WORK WAS COMPETED IN CONFORMANCE WITH THE PLANS.	Mean Horizontal Equal Hazard Spectra East End of Train Box (475 Return Period) FINAL STAGE - TEMPORARY BRIDGE DESIGN	SIONS PPTION PPTION BRIDGE SIJBHITAL BRIDGE SIJBHITAL MIERERENCE SIJBHITAL MIERERENCE SIJBHITAL DE COMMENTS E TO C
RESPONSE SPECINA C.7. RESPONSE TO RFI T-0209.3 DATED SEP. 28, 2011: BSE - ABUTMENT BEARING ON COSM WALL - FOLLOW-UP	B.5.DENSITY: MATERIALS SHALL BE PROPORTIONED TO PRODUCE CONCRETE WITH A CALCULATED EQUILIBRIUM DENSITY OF 110 PCF ± 3 PCF AS DETERMINED BY ASTM C567-00, SECTION 9.2	SPECIFICATIONS, AND THE APPLICABLE WORKMANSHIP PROVISIONS OF THE CBC.	1.2	CEVISI EMP. B EMP. B FONDE PONSE STREE
C.8. RESPONSE TO RFI T-0219 DATED SEP. 14, 2011: BSE - ABUTMENTS AT TEMPORARY BRIDES	C. STRUCTURAL STEEL SHALL BE NEW EXCEPT THE W14 GIRDERS AND CAP BEAM FOR THE END SPANS AND IT SHALL MEET OR EXCEEDS THE	A.3.ITEMS REQUIRING SPECIAL INSPECTIONS; A.3.1. INSPECTION OF MILL CERTIFICATES OR LAB RESULTS FOR ALL		REVISION DESCRIPTION DESCRIPTION T555 TEMP. BRID PER REVERSE OF STRUT INTER STRUT INTER ESPONSE TO RESPONSE TO RESPONSE TO RESPONSE TO RESPONSE TO RESPONSE TO RESPONSE TO RESPONSE TO RESPONSE TO RESPONSE TO
C.9. RESPONSE TO RFI T-0222 DATED SEP. 01, 2011: BSE - TEMPORARY BRIDGE PIER LOCATIONS	FOLLOWING REQUIREMENTS	COMPONENTS OF THE TEMPORARY BRIDGES.	5 0.8 C	80%
C.10. RESPONSE TO RFI T-0223 DATED SEP. 27, 2011: BSE - TEMPORARY	C.1.STEEL SHAPES: ASTM A992, GRADE 50 Fy = 50 KSI	A.3.2. AWS PREQUALIFIED WELDING PROCEDURES, OR DOCUMENTATION OF QUALIFICATION PERFORMED FOR PROJECT SPECIFIC WELDING PROCEDURES IN ACCORDANCE WITH AWS	S ON	
BRIDGE PEDESTRIAN BARRIER HEIGHT	C.2.STEEL REINFORCEMENT: ASTM A615, GRADE 60 Fy = 60 KSI	D1.5 REQUIREMENTS.	2¥ 0.4	DATE 65/10/11 66/16/11 06/06/11 10/06/11 10/06/12 65/07/12 65/07/12 65/07/12
C.11. RESPONSE TO RFI T-0228 DATED SEP. 27, 2011: BSE - 6-INCH SIDEWALK AT TEMPORARY BRIDGES	C.3.STEEL PLATES: ASTM A572, GRADE 50 Fy = 50 KSI	A.3.3. TESTING OF STRUCTURAL WELDS	0.2	0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0
C.12. PROJECT SPECIFICATION ADDENDUM NO. 3 SECTION 01 53 13 - TEMPORARY BRIDGES DATED SEPTEMBER 23, 2010	C.4.STEEL ANGLES: ASTM A36, GRADE 36 Fy= 36 KSI, U.N.O. C.5.STEEL PIPE SECTIONS FOR CONNECTION BETWEEN CIDH PILE AND CAP	A.3.4. INSTALLATION OF ALL A490-X BOLTS FASTENERS (UNLESS "TENSION CONTROLS" BOLTS ARE USED) INCLUDING		
C.13. CAD FILES RECEIVED IN RESPONSE TO RFI T-0035.1 DATED MAR. 23, 2011: BSE - REQUEST FOR STRUCTURE SECTION DRAWINGS	BEAM: ANSI B36.1/ASTM A53, FY = 35 KSI.	INSPECTION AND ACCEPTANCE OF BOLT HOLES PRIOR TO INSTALLATION OF FASTENERS. INSPECTION IS TO VERIFY PROPER PREPARATION OF HOLES, SPECIALLY NO OVERSIZED	PERIOD (SEC)	AM AM
CODES AND STANDARDS	C.6.STEEL BOLTS: ASTM A490-X BOLTS, MIN. TENSILE STRENGTH = 150 KSI	HOLES OR FLAME CUTTING HAS BEEN USED TO INCREASE TOLERANCES TO ACHIEVE FIELD FIT.		
STRUCTURAL PERFORMANCE: BRIDGES SHALL BE PROVIDED MEETING DESIGN LOADING CRITERIA COMPLIANT WITH THE	C.7.STANDARD MILD STEEL STUDS MANUFACTURED BY NELSON CONFORM	A.3.5. VERIFICATION THAT EXCAVATIONS ARE EXTENDED TO THE		
CURRENT AMERICAN ASSOCIATION OF STATE HIGHWAY AND TRANSPORTATION OFFICIALS (AASHTO) LRFD DESIGN	TO ASTM A108 SPECIFICATIONS FROM 1010 THROUGH 1020 MILD STEEL.	PROPER DEPTH. A.3.6. PLACEMENT OF CONCRETE		CA CA CA ORARY
SPECIFICATIONS, 4TH EDITION, AND CALTRANS AMENDMENTS.	C.8.WELDED WIRE REINFORCEMENT: ASTM A185, GRADE 65	A.S.O. PLACEMENT OF CONCRETE A.4.CONTRACTOR SHALL INSPECT BRIDGE PAVEMENT FOLLOWING USAGE		
A.1. LIMIT STATE DESIGN A.1.1. STRENGTH I - DC+DW+HL-93 DESIGN VEHICLE W/ 33% IM	C.9.STRUCTURAL METAL DECK: ASTM A653 GR. 50, 4 ∛ DEEP, MAX ALLOWABLE CLEAR SPAN BETWEEN GIRDER FLANGES FOR HL-93 &	FOR TRAVEL BY CONTRACTOR EQUIPMENT. ALL DAMAGE OR SURFACE IRREGULARITIES RESULTING FROM SUCH USAGE SHALL BE IMMEDIATELY		¥ °∪∐ S
A.1.2. STRENGTH II - DC+DW+CALIFORNIA PERMIT TRUCK P15 W/ 25% IM	CALIFORNIA PERMIT TRUCK P15 LOADING OF 57 INCHES AND FOR MANITOWOC 999 SERIE 2 CRANE OF 26 INCHES DESIGNED IN	CORRECTED.		
A.1.3. SERVICEABILITY AND FATIGUE CHECKS	ACCORDANCE WITH SECTION 1.2 CODES AND STANDARDS. THE MANUFACTURER IS TO PROVIDE THE CERTIFICATION FOR THE REFERENCED LOADING.	A.5. APPROACH TRANSITIONS FROM ROAD TO BRIDGES SHALL BE INSPECTED FREQUENTLY FOR SETTLEMENT. REPAIR WITH HOT ASPHALT CONCRETE PAVEMENT AS NECESSARY.		
A.1.4. CONSTRUCTION LOADS W/ MANITOWOC-999 SERIES 2 CRANE LOADING WITHOUT ANY HOOK LOAD (WITHIN DESIGNATED AREAS ONLY)	C.10.ALL WELDS SHALL BE MADE USING LOW-HYDROGEN ELECTRODES WITH MINIMUM TENSILE STRENGTH = 70 KSI. SEE SPECIFICATIONS FOR	A.6. CONTRACTOR SHALL ON A MONTHLY BASIS INSPECT AND REPAIR SURFACE IRREGULARITIES IN BRIDGE PAVEMENT OCCURRING IN		
A.2. SEISMIC DESIGN	FURTHER REQUIREMENTS. C.11.IN COMPLIANCE TO WELDING PROCEDURES AND SPECIFICATIONS' THE	VEHICLE AND PEDESTRIAN AREAS. A.7. SPECIAL INSPECTION ITEMS OF METAL DECK	DRAWING INDEX	
A.2.1. BASED ON CALIFORNIA SEISMIC DESIGN CRITERIA (SDC), VERSION 1.6, NOVEMBER 2010	FOLLOWING PROCEDURES AND/OR PREPARATIONS ARE TO BE FOLLOWING PROCEDURES AND/OR PREPARATIONS ARE TO BE FOLLOWED:	PERIODIC INSPECTIONS SHALL BE PERFORMED TO CONFIRM THE PERFORMANCE OF THE METAL DECK WELDS WITH FATIGUE LOADING		ANS ANS ANS ANS ANS
A.2.2. DISPLACEMENT DEMANDS ARE CALCULATED BASED ON STRUCTURAL FUNDAMENTAL PERIOD AND DESIGN	C.11.1. JOINT PREPARATIONS AND WELDING PROCEDURES INCLUDING	THE INSPECTION SHALL CONSIST OF DYE PENETRANT, MOREVETIC PARTICLE OR OTHER NON-DESTRUCTIVE METHOD AS APPROPRIATE FOR THE CONFIGURATION OF THE WELD AND THE DECREE OF	SH-0100 GENERAL NOTES AND DRAWING INDEX SH-0101 LEGENDS, SYMBOLS AND ABBREVIATIONS	
ACCELERATION RESPONSE SPECTRA.	BUT NOT LIMITED TO: WELDING PROCEDURES, REQUIRED ROOT OPENINGS, ROOT FACE DIMENSIONS, GROOVE ANGLES, BACKING BARS, COPES, SURFACE ROUGHNESS VALUES, AND	PREPARATION, TO CONFIRM THAT FATIGUE CRACKS ARE NOT FORMING. ON THE TWO MOST HEAVILY TRAVELED BRIDGES, FIRST STREET AND	SH-1100 CONSTRUCTION SEQUENCE	
A.2.3. DISPLACEMENT CAPACITY IS BASED ON DUCTILITY OF THE STRUCTURE AND DETERMINED BY PUSH-OVER	WELDING TAPERS OF UNEQUAL PARTS.	FREMONT STREET, THREE INSPECTIONS WITHIN THE RIGHT LANE (ASSUMED TO BE THE MOST HEAVILY TRAVELED BY TRUCKS) SHALL	SH-1101 CRANE TRAVEL LAYOUT	→ zưià
ANALYSIS IN BOTH TRANSVERSE AND LONGITUDINAL DIRECTION.	C.11.2. SEQUENCING AND PROCEDURES OF WELDING TO MINIMIZE THE EFFECT OF SHRINKAGE, RESIDUAL STRESSES, AND TO	BE IDENTIFIED FOR TESTING. THESE LOCATIONS SHALL BE AS FOLLOWS:	SH-2100 PARTIAL PLAN FIRST STREET BRIDGE SH-2101 PARTIAL PLAN FIRST STREET BRIDGE	FRE RA
STANDARD SPECIFICATIONS 2006, CALTRANS	MAINTAIN ERECTION TOLERANCES.	1. DIRECTLY OVER A STEEL GIRDER, DIRECTLY OVER THE BENT CAP	SH-2101 PARTIAL PLAN FIRST STREET BRIDGE SH-2102 PARTIAL PLAN FREMONT STREET BRIDGE	
C. AMERICAN WELDING SOCIETY, AWS D1.5: 2010 'BRIDGE WELDING CODE", 6TH EDITION	D. ASPHALT CONCRETE D.1.ASPHALT CONCRETE SHALL BE TYPE A UNLESS OTHERWISE SPECIFIED	2. DIRECTLY OVER A STEEL GIRDER, CENTERED BETWEEN BENT CAPS	SH-2103 PARTIAL PLAN FREMONT STREET BRIDGE	AN AN
 AMERICAN INSTITUTE OF STEEL CONSTRUCTION (AISC) STEEL DESIGN MANUAL, 13TH EDITION 	BY THE PROJECT SPECIFICATIONS.	 IF THERE IS A LOCALIZED DETERIORATION OF ASPHALT, AT A SITE OF DETERIORATION. IF NO DETERIORATION, AT A RANDOM LOCATION. 	SH-2104 PARTIAL PLAN BEALE STREET BRIDGE SH-2105 PARTIAL PLAN BEALE STREET BRIDGE	ZL F
2 - CONSTRUCTION	D.2. ASPHALT BINDER SHALL BE PERFORMANCE GRADE 64-10 PAVING ASPHALT CONFORMING TO SECTION 92, "ASPHALT", OF THE CALTRANS	AT EACH IDENTIFIED LOCATION, AN AREA OF APPROXIMATELY 1-FOOT	SH-2105 PARTIAL PLAN BEALE STREET BRIDGE SH-3100 TEMPORARY BRIDGE KEY ELEVATION	
CONSTRUCTION PROCEDURE	STANDARD SPECIFICATIONS UNLESS OTHERWISE SPECIFIED ON THE PROJECT SPECIFICATIONS.	BY 2-FOOT 6-INCHES OF ASPHALT SHALL BE REMOVED TO EXPOSE AT LEAST THREE WELDS FOR TESTING. IF FATIOLE CRACKS ARE FOUND, ADDITIONAL TESTING MAY BE NECESSARY TO VERIFY THE EXTENT OF	SH-3101 TRANSVERSE ELEVATION FIRST STREET BRIDGE	
A. CONSTRUCTION PROCEDURE FOR THE CDSM WALLS WILL BE COVERED UNDER OTHER SUBMITTALS.	D.3.THE PERCENTAGE OF AIR VOIDS IN THE MIX DESIGN AT THE TARGET ASPHALT BINDER CONTENT SHALL BE BETWEEN 3% AND 5%.	CRACKING AND POTENTIAL REPAIRS. THE FHA MANDATED TWO YEAR INSPECTION FREQUENCY IS APPROPRIATE FOR THIS STRUCTURE	SH-3102 TRANSVERSE ELEVATION FREMONT STREET BRIDGE	-01
B. CONSTRUCTION PROCEDURE FOR INSTALLATION OF TEMPORARY BRIDGES.	D.4. AGGREGATE USED IN ASPHALT CONCRETE SHALL CONFORM TO THE	INCREASE INSPECTION FREQUENCY IF ACP DETERIORATION IS OBSERVED. TUPA AND/OR DESIGNATED REPRESENTATIVE CAN REQUEST INSPECTION IF THERE IS EVIDENCE OF POTENTIAL DECK WEDING FAILURE.	SH-3003 TRANSVERSE ELEVATION BEALE STREET BRIDGE SH-3104 BRIDGE DECKING SECTIONS GRID LINES & BENTS	153
B.1. SEE SHEET SH-1100 FOR THE CONSTRUCTION PROCEDURE FOR THE	GRADING REQUIREMENTS OF SECTION 39-2.02, "AGGREGATE", OF THE STANDARD SPECIFICATIONS.	IF THERE IS EVIDENCE OF POTENTIAL DECK WELDING FAILURE. B. SITE OBSERVATION	SH-3105 BRIDGE DECKING SECTIONS ABUTMENTS ON THE NORTH SIDE	313
 INSTALLATION OF THE TEMPORARY BRIDGES. B.2. GENERAL CONTRACTOR SHALL RESOLVE ANY CONFLICT BETWEEN BRIDGE COLUMNY/PIER/BRACING, STRUTZ AND STRUCTURAL COLUMNS/PILE CAPS 	D.5. AREA SUBJECT TO VEHICLE TRAVEL SHALL BE SURFACES PROVIDING A MINIMUM STATIC COEFFICIENT OF FRICTION EQUAL TO 0.65.	B.1. REPRESENTATIVE OF PB&A SHALL VISIT THE SITE AT LEAST ONCE A WEEK DURING TEMPORARY BRIDGE INSTALLATION AND EXCAVATION	SH-3106 BRIDGE DECKING SECTIONS ABUTMENTS ON THE SOUTH SIDE SH-4100 LONGITUDINAL SECTIONS FIRST & FREMONT STREET BRIDGES	PRINCIPAL ENGINEER
COLOMMY PIERY BRACING, SINUS AND SINUCURAL COLOMNS/PILE CAPS PRIOR TO START OF CONSTRUCTION. C. PROCEDURE FOR INSTALLATION OF BRIDGE COLUMN PIERS	D.6. AREA SUBJECT TO PEDESTRIAN TRAVEL SHALL BE SURFACES PROVIDING A MINIMUM STATIC COEFFICIENT OF FRICTION EQUAL TO	OPERATIONS TO PROVIDE STRUCTURAL OBSERVATION IN ACCORDANCE WITH THE SAN FRANCISCO BUILDING CODE. PB&A SHALL ISSUE A WRITTEN REPORT FOR EACH SITE VISIT.	SH-4101 LONGITUDINAL SECTION BEALE STREET BRIDGE	PROJECT MANAGER P.
C.1. DRILL 48"Ø HOLE FROM GROUND LEVEL TO THE DESIGNED TIP ELEVATION.	0.65 WHERE SLOPES ARE LESS THAN OR EQUAL TO 5%, AND 0.80 WHERE SLOPES EXCEED 5%.	C. MONITORING	SH-5100 DETAILS SH-5101 DETAILS	Q. LIU P. BARAR
A 60"# X 20'-0" "STARTER" CASING IS USED IN THE UPPER 20 FT OF	E. RAILING	C.1.PLACE A SURVEY MONITORING POINT ON THE BRIDGE DECK AT EACH	SH=5102 DETAILS	Q. LIU P. BARAR DRAW BY: ML/ PS/ CP 05/02/2011 SOLE SET FAGUY NO.
LOOSE MATERIAL. HOLE IS SUPPORTED BY DRILLING SLURRY (WATER AND				
LOOSE MATERIAL. HOLE IS SUPPORTED BY DRILLING SLURRY (WATER AND POLYMER). C.2. VERIFY THE TIP ELEVATION MANUALLY ONCE IT IS REACHED.	E.1. THRIE BEAM: AASHTO M180, CLASS A, TYPE II (ZINC) COATING	BENT. C.2.OBTAIN THE READINGS WEEKLY DURING ACTIVE EXCAVATION. KEEP	SH-5103 DETAILS SH-5104 DETAILS	AS NOTED E 140 SHEET NUMBER SEQUENCE

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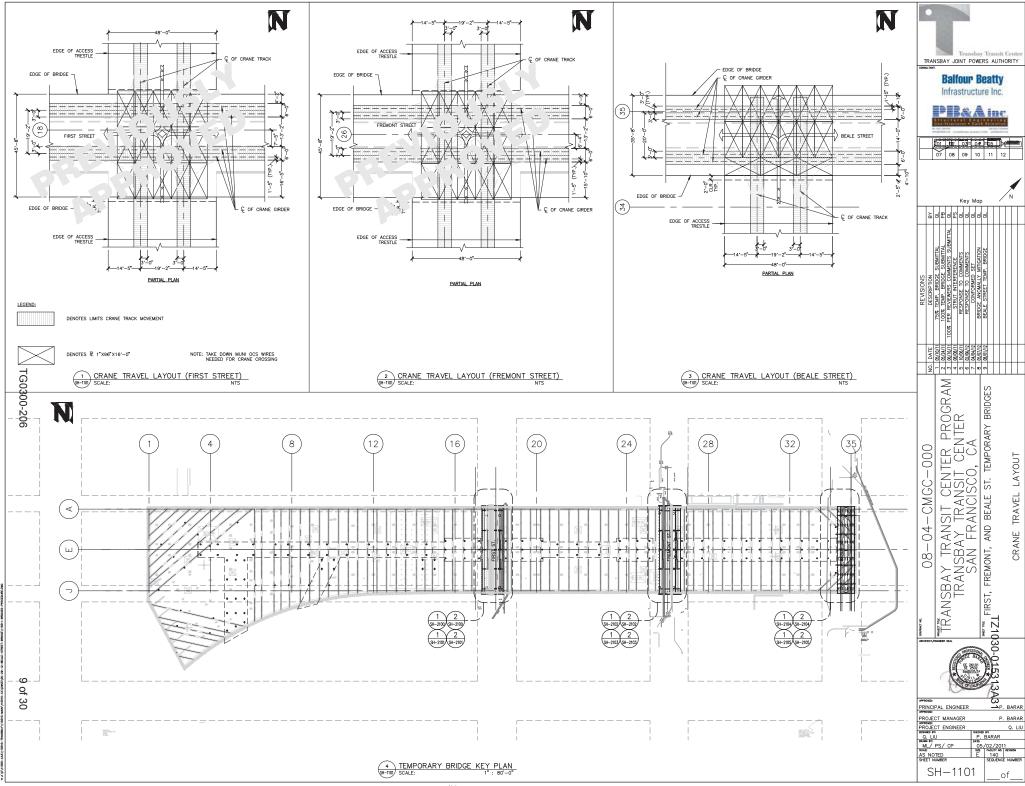
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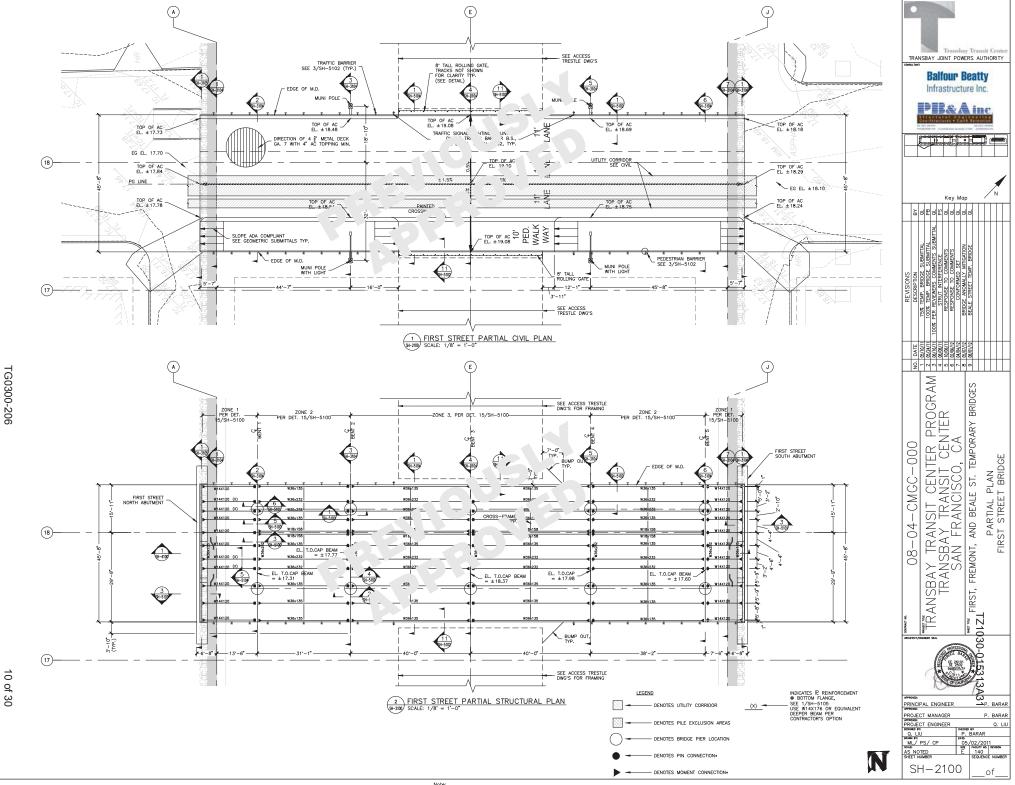




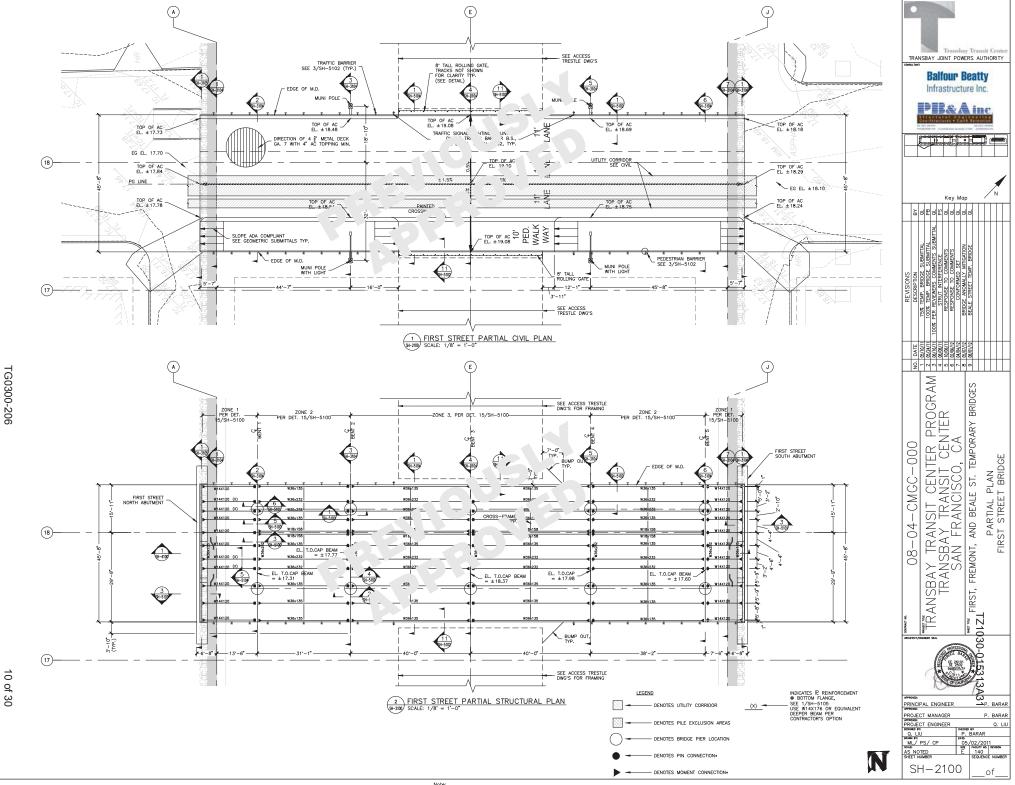
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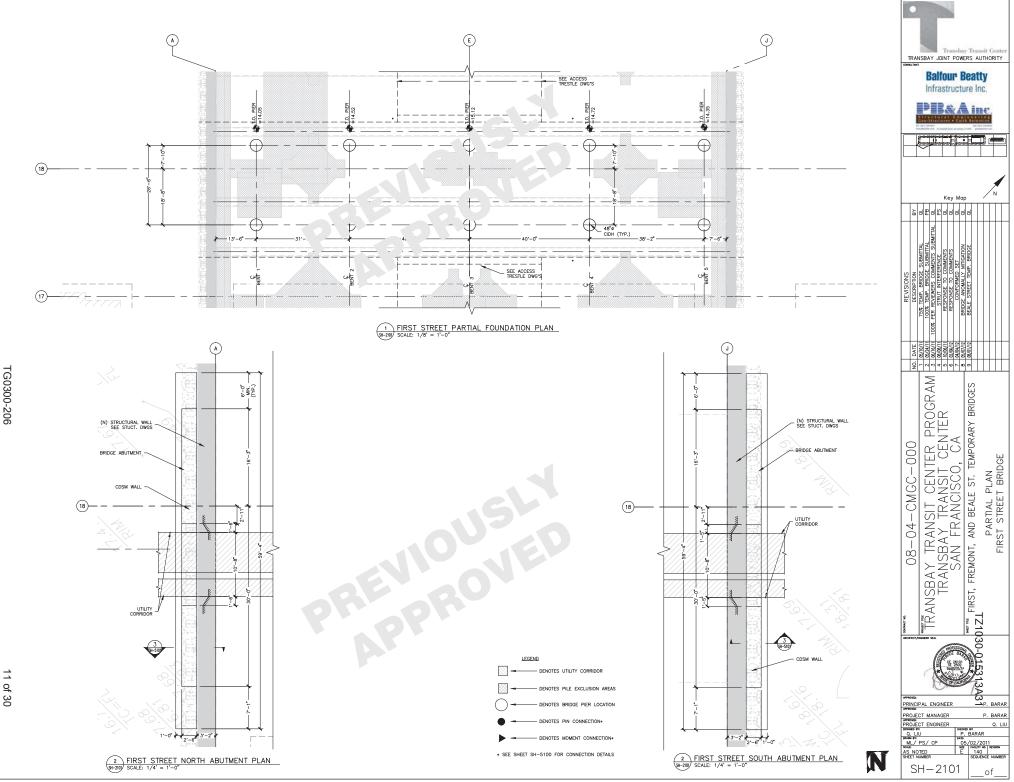
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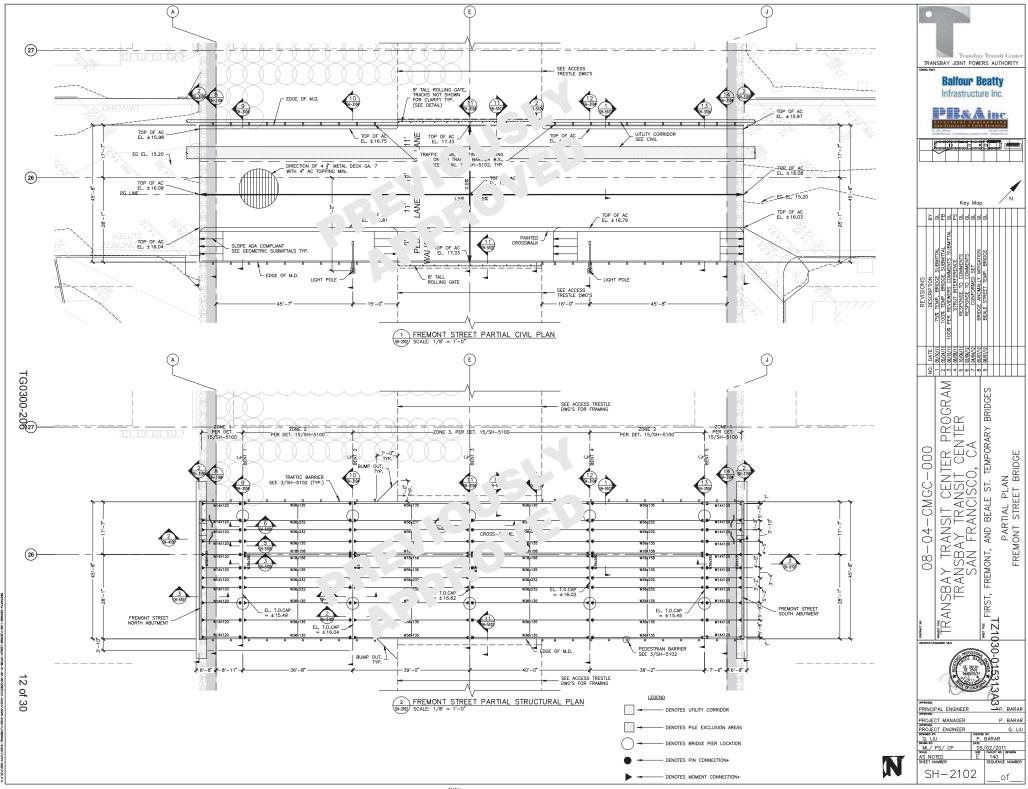
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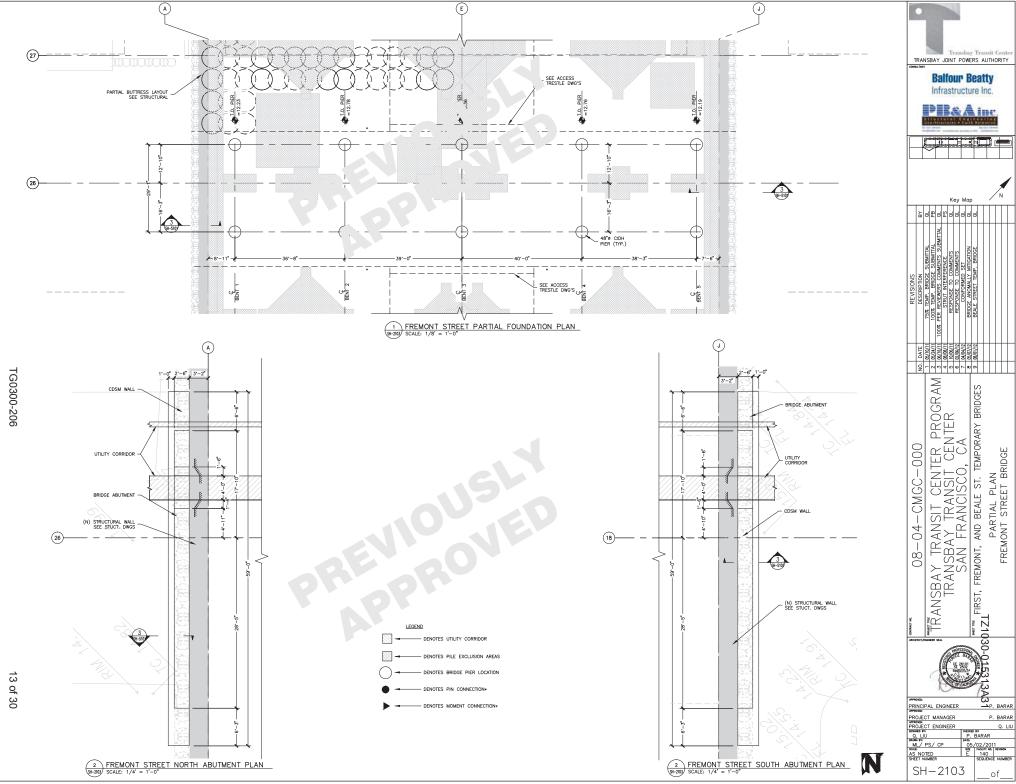
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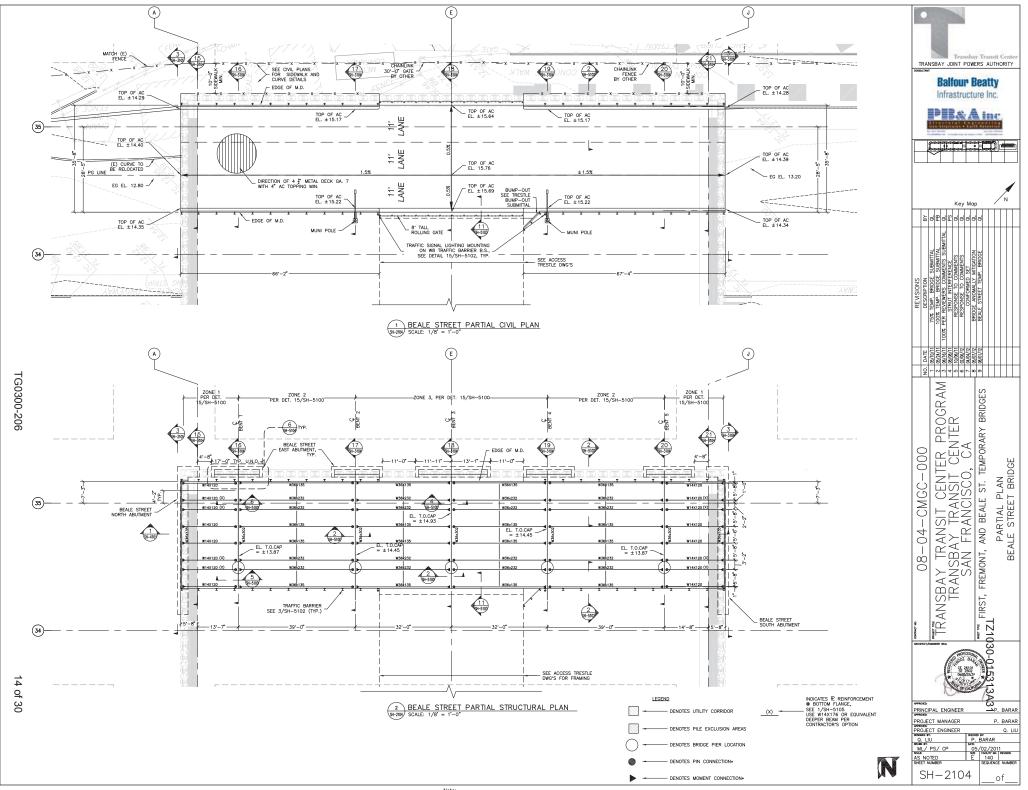
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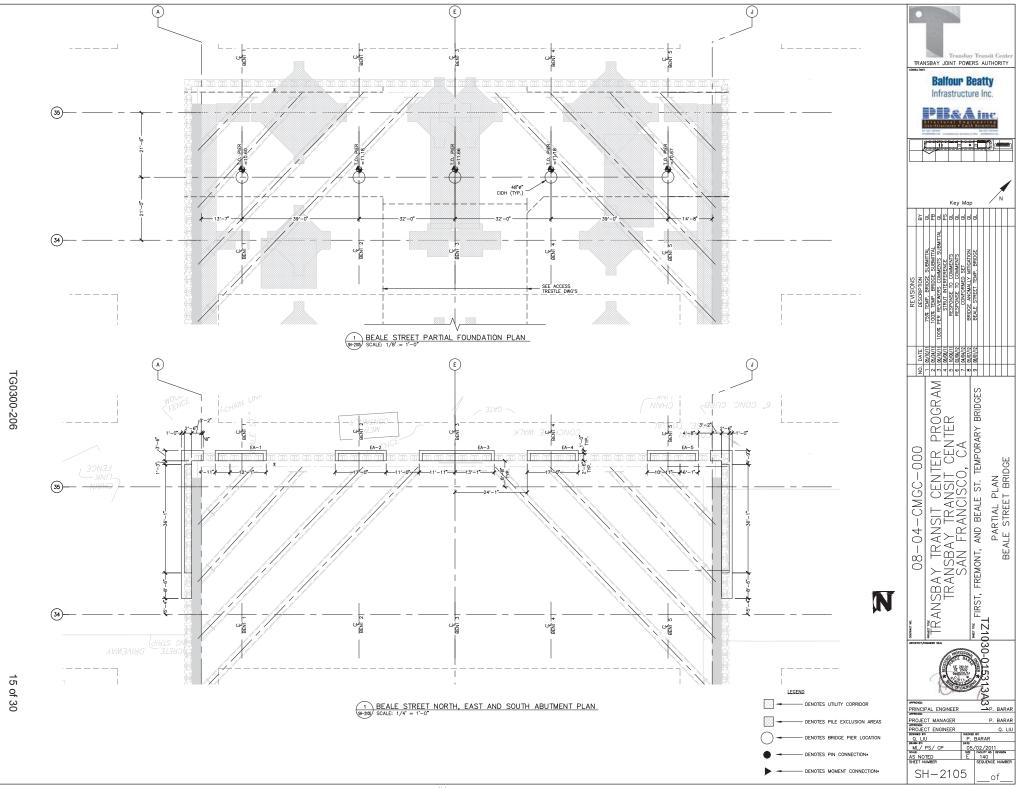
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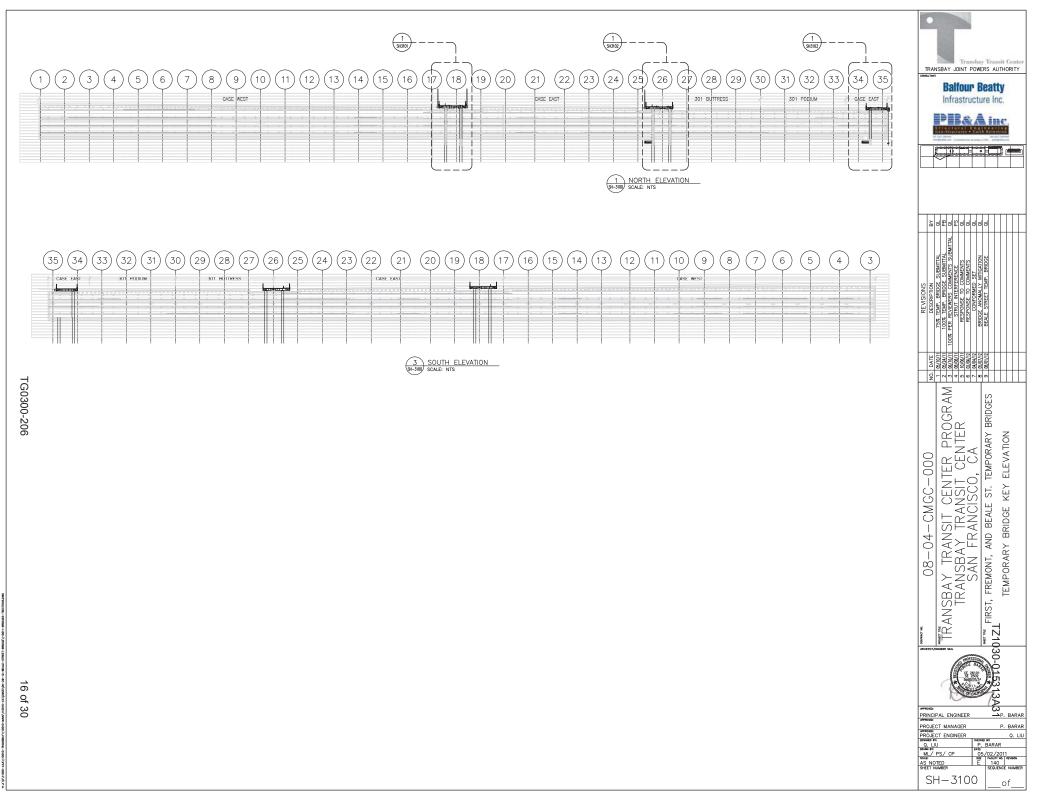


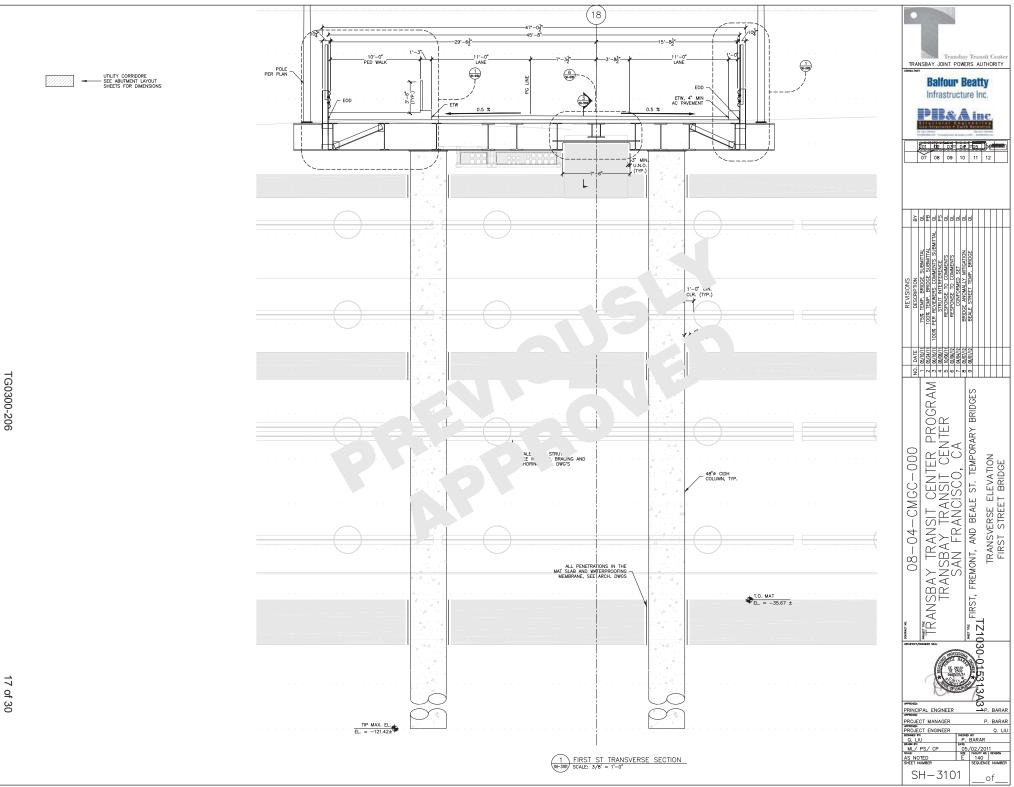
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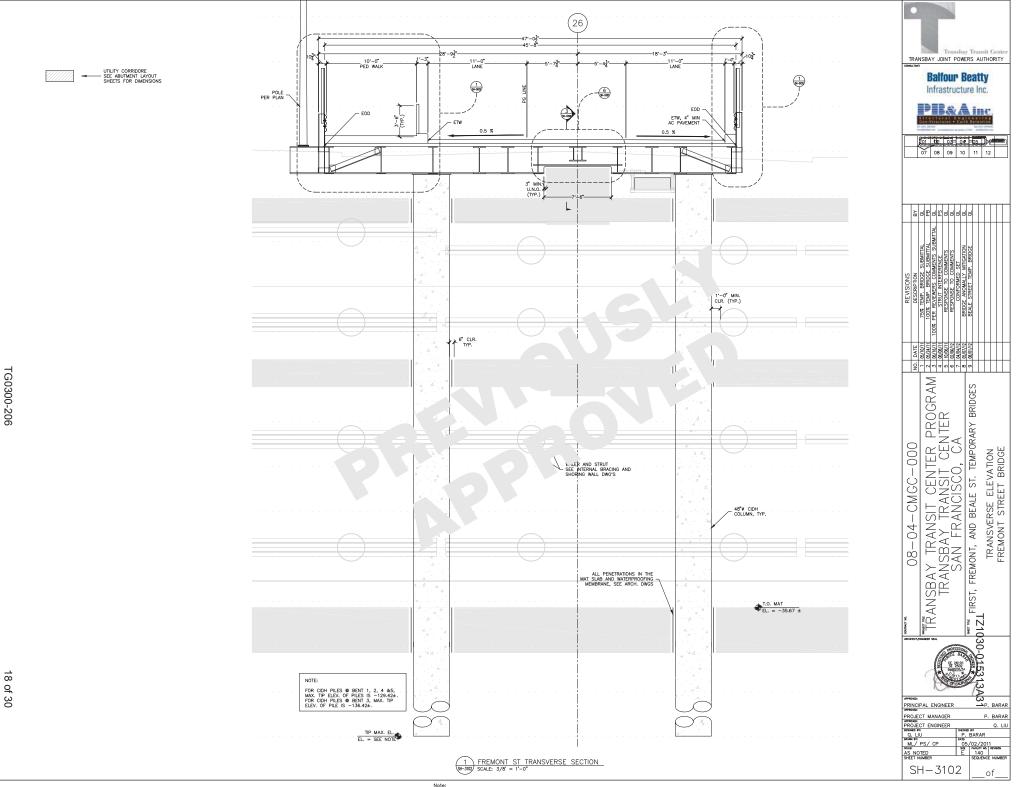
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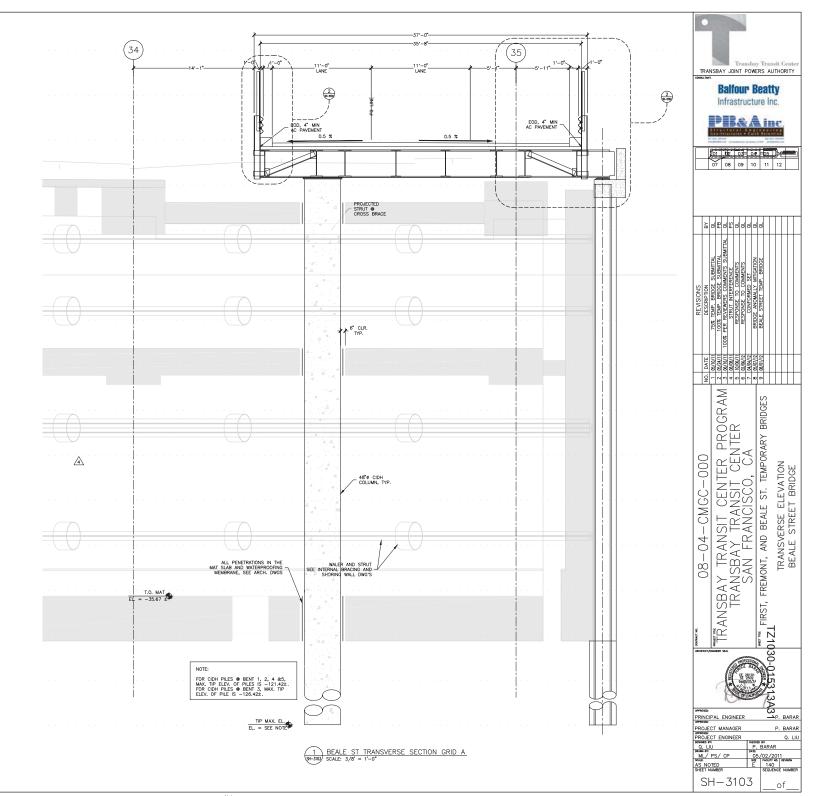
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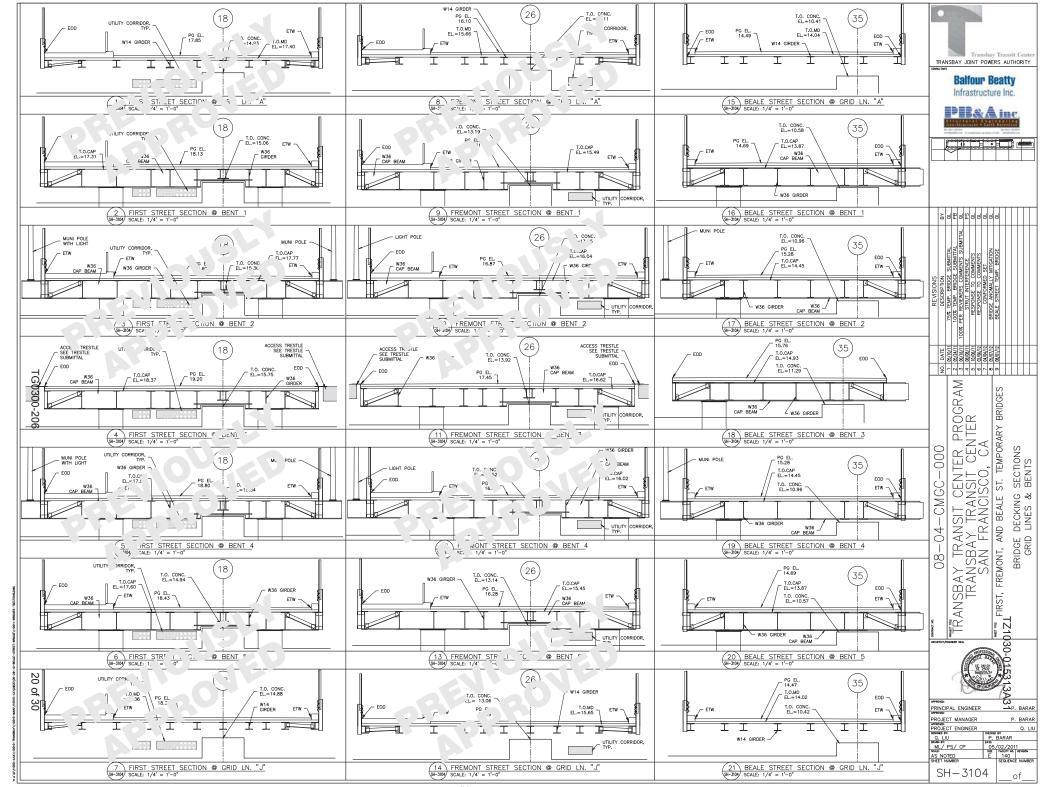


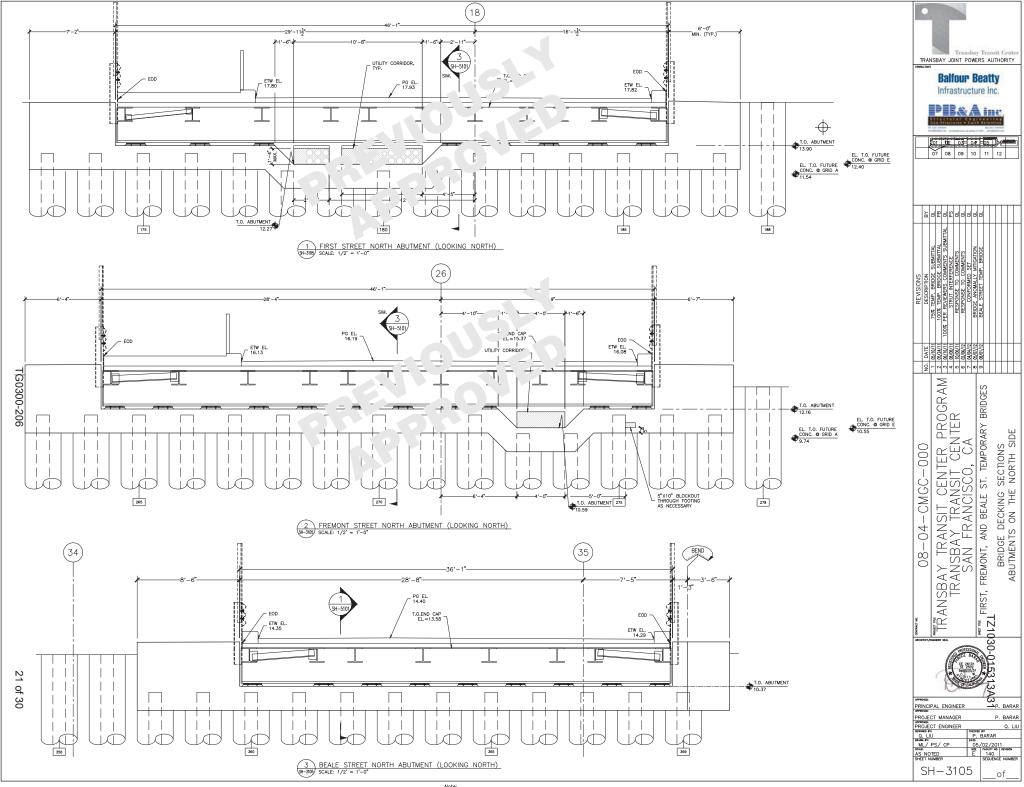
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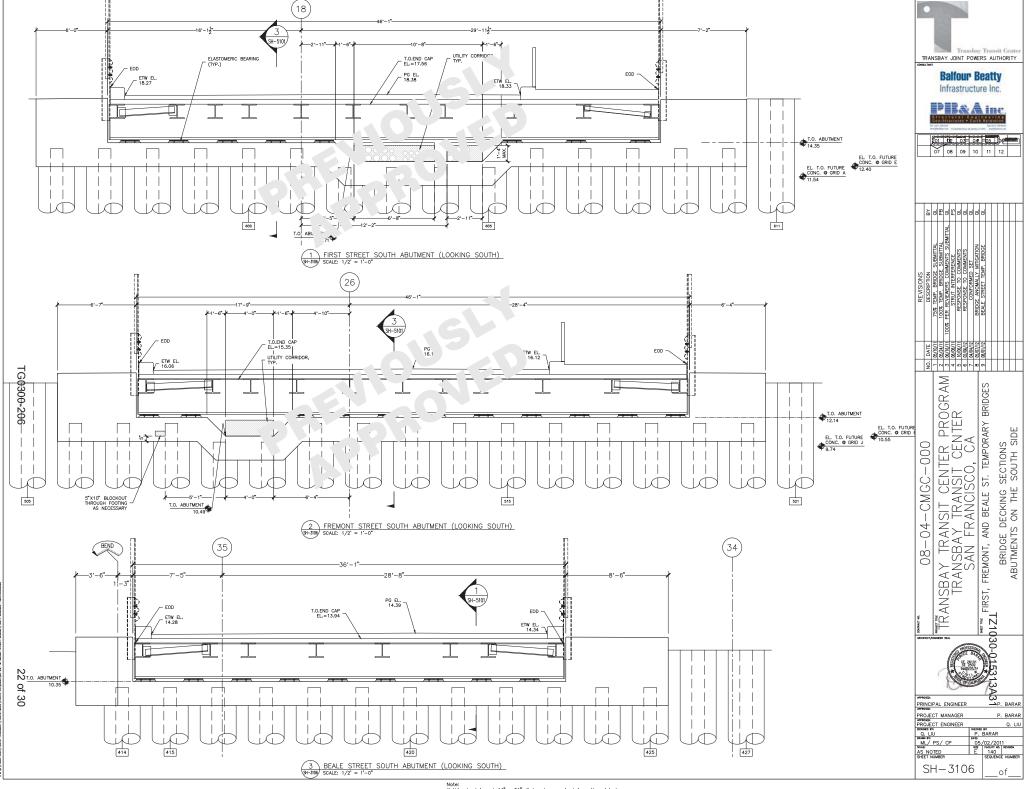
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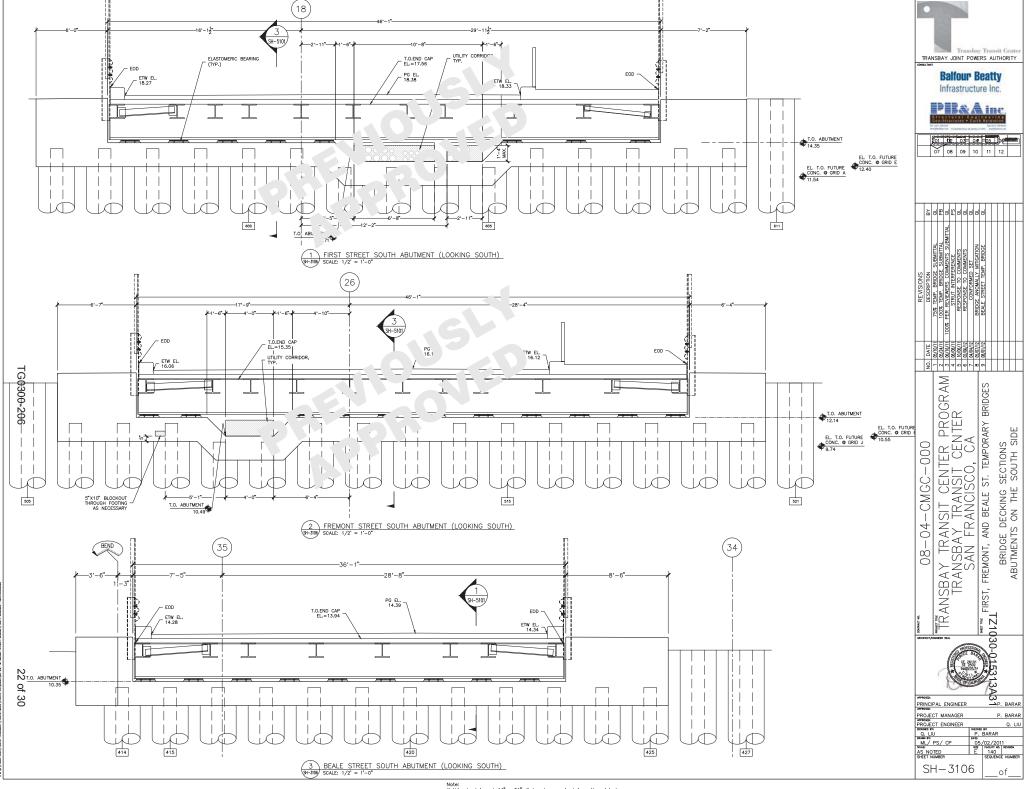


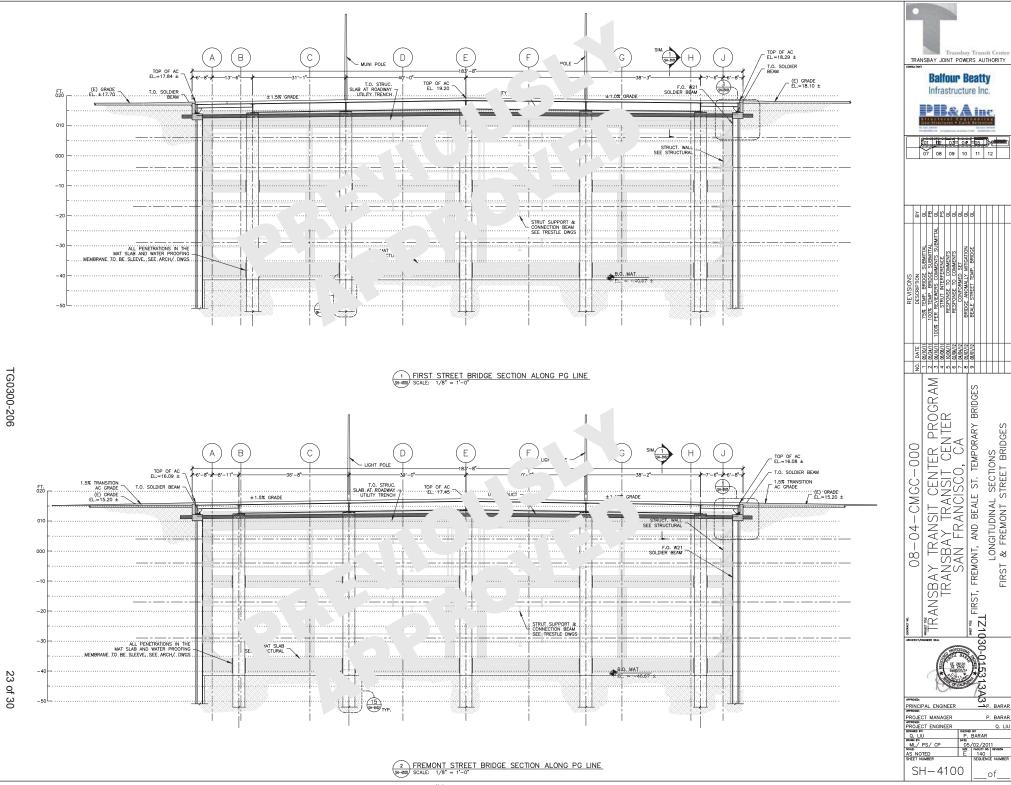




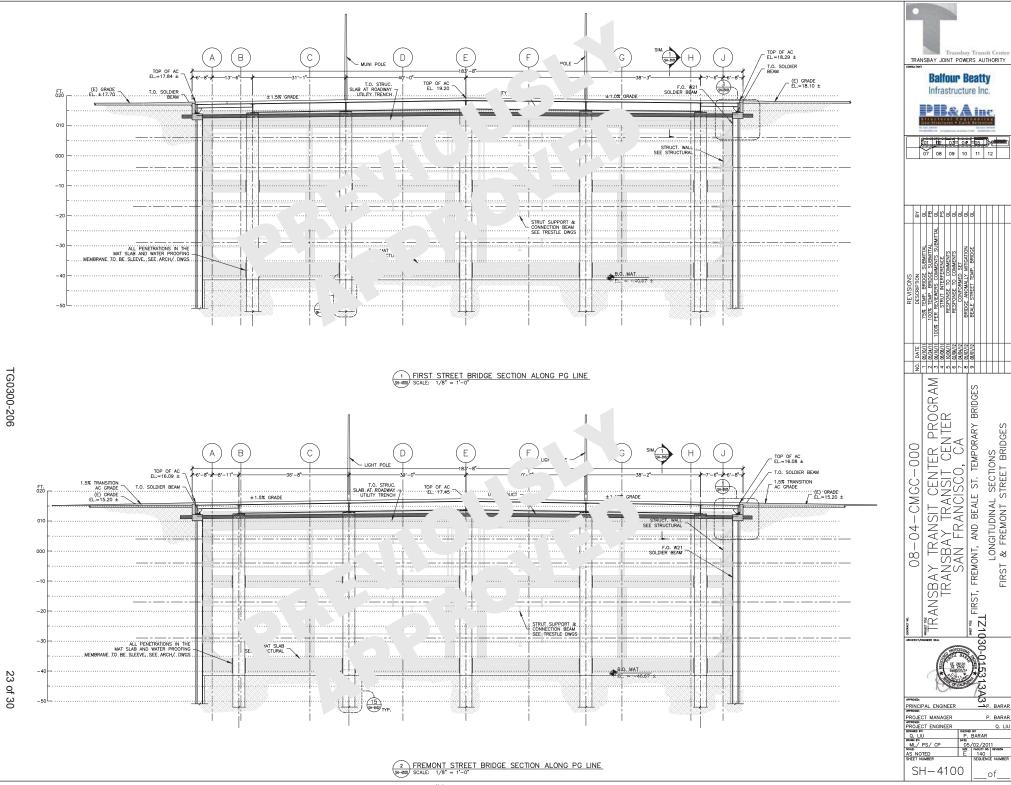




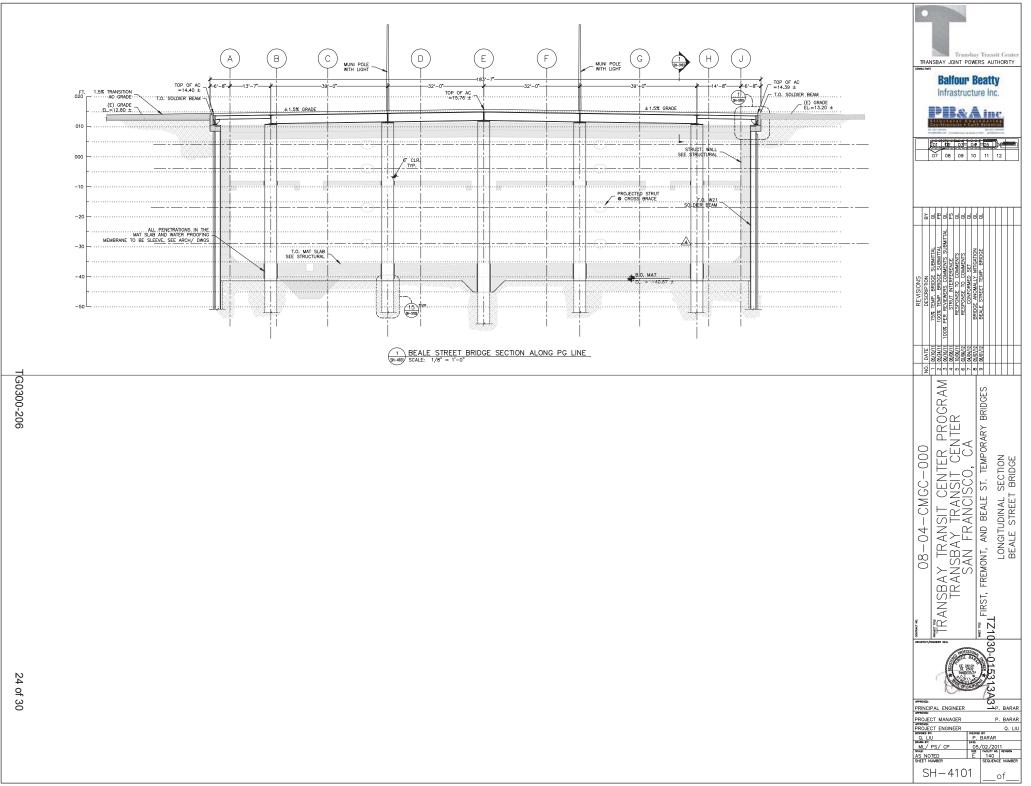


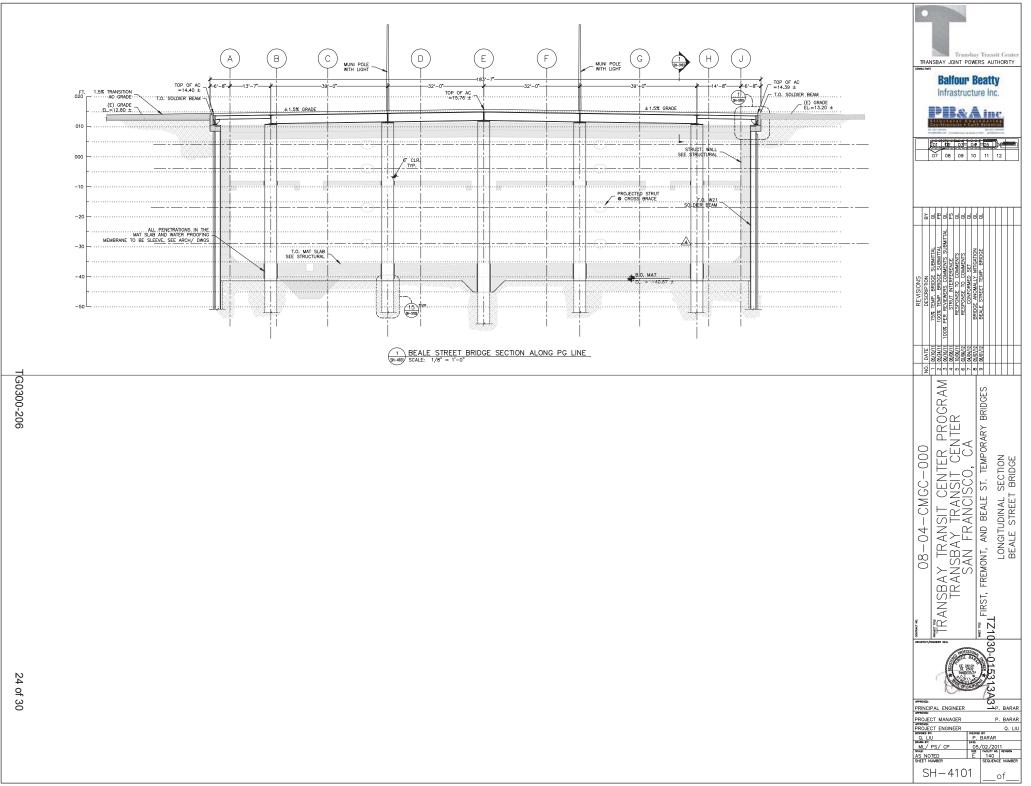


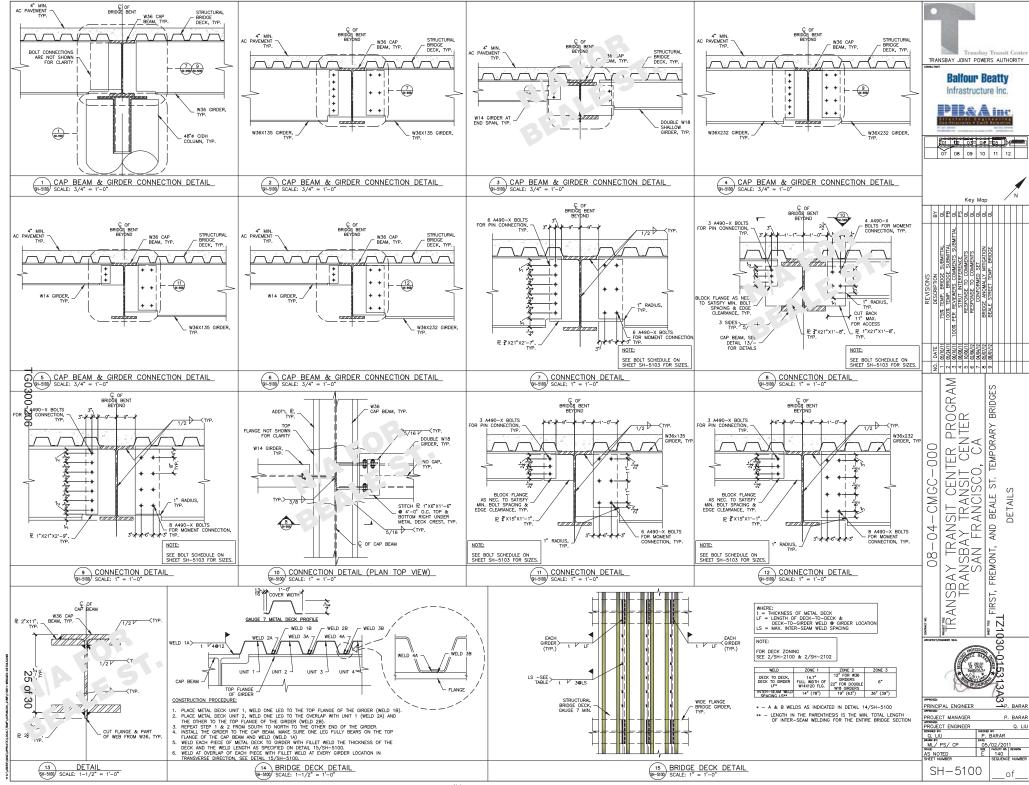
Note: If this sheet is not $44^* \times 34^*$, it has been revised from its original size. Scales noted on drawings/details are no longer applicable.



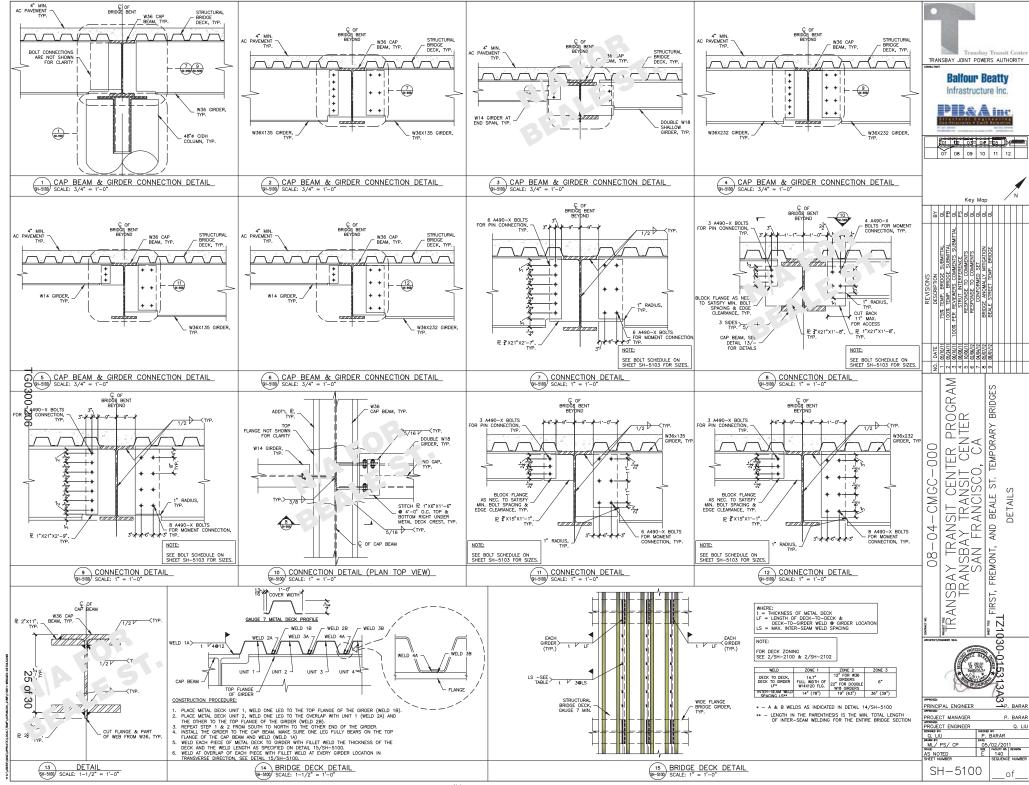
Note: If this sheet is not $44^* \times 34^*$, it has been revised from its original size. Scales noted on drawings/details are no longer applicable.



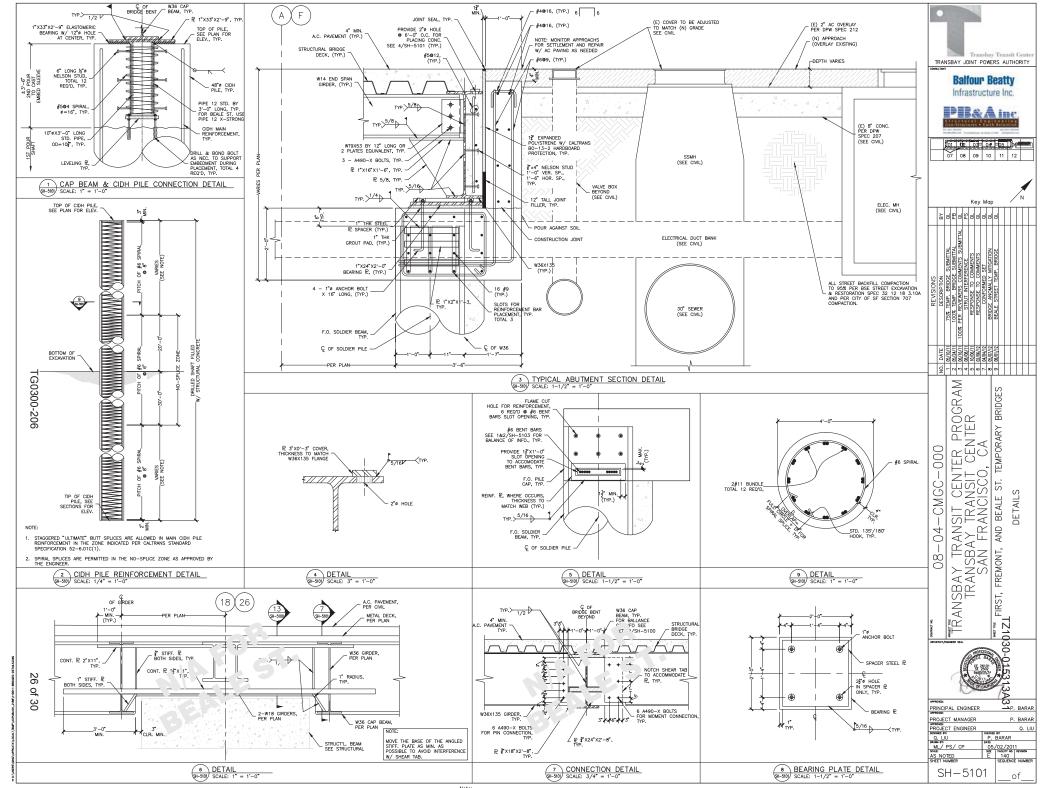


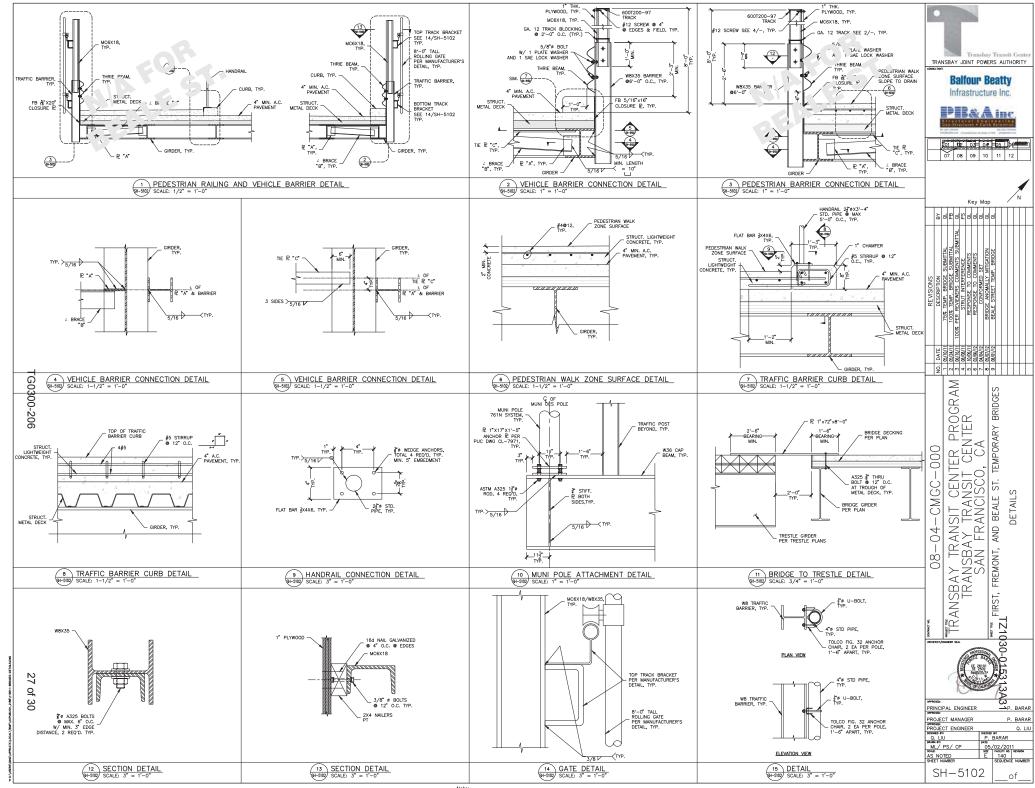


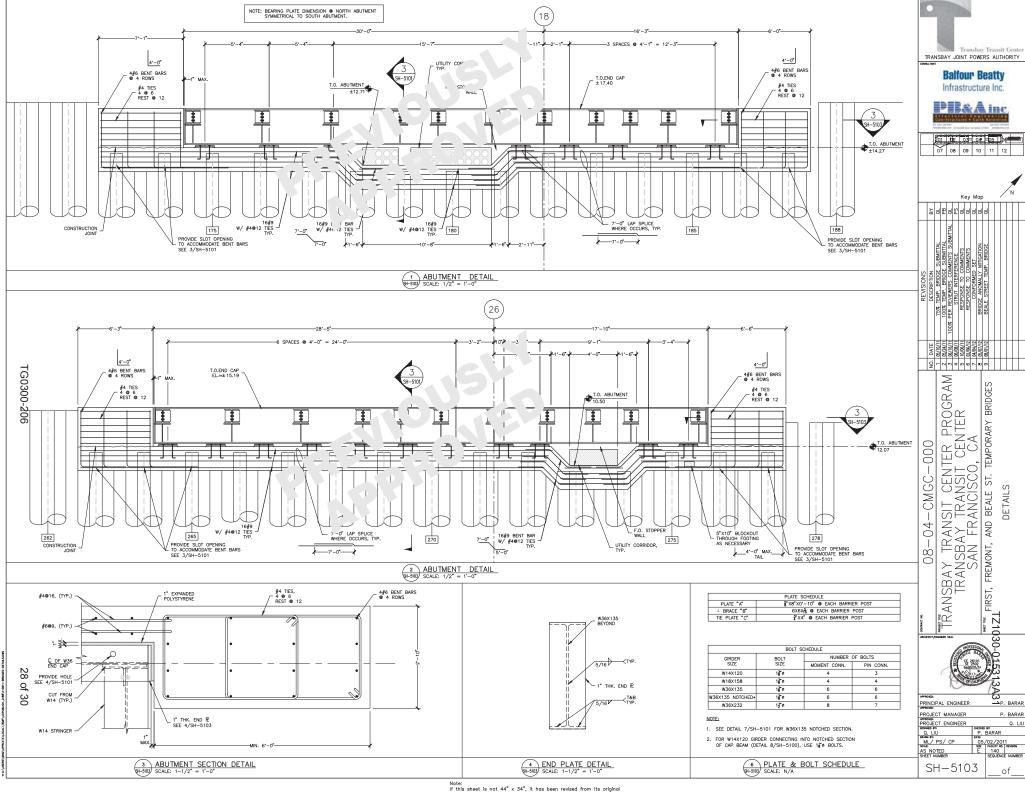
Note: If this sheet is not 44" x 34", it has been revised from its original size. Scales noted on drawings/details are no longer applicable.



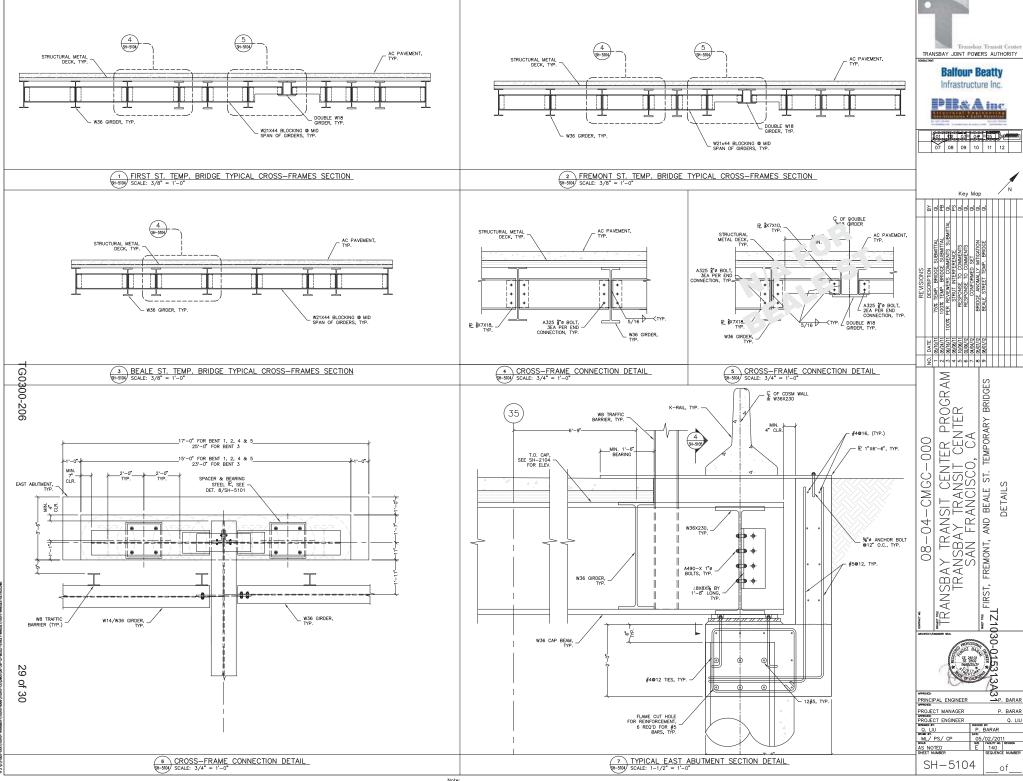
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